CODE ANALYSIS

APPLICABLE CODES

	Year		Year
International Building Code	2003	National Electrical Code	2003
International Mechanical Code	2003	Uniform Code for	
International Plumbing Code	2003	Building Conservation	
International Fire Code	2003	ADA Accessibility	
International Energy Conservation Code	2003	Guildelines	ICC/ANSI 117.1 1998

A. Occupancy and Group: UNLIMITED MALL BUILDING

Change in Use: Yes _____ No ✓ Mixed Occupancy: Yes ✓ No _____
Special Use and Occupancy (e.g. High Rise, Covered Mall): UNLIMITED MALL BUILDING

B. Seismic Design Category: D Design Wind Speed: 90 mph

C. Type of Construction (circle one):

I A | I B | II A | (II B) | III A | III B | IV HT | V A | V B

D. Fire Resistance Rating Requirements for the Exterior Walls based on the fire separation distance (in hours):

North: _____ South: _____ East: _____ West: _____

E. Mixed Occupancies: _____ Nonseparated Uses: _____

F: Sprinklers:

Required: ✓ Provided: ✓ Type of Sprinkler System: _____

G: Number of Stories: 1 Building Height: 18'

H: Actual Area per Floor (square feet): 1800 SF

I: Tabular Area: UNLIMITED MALL BUILDING

J: Area Modifications:

a) $A_a = A_t + \left[\frac{A_t I_f}{100}\right] + \left[\frac{A_t I_s}{100}\right]$ $\qquad I_f = 100\left[\frac{F}{P} - 0.25\right]\frac{W}{30}$

b) Sum of the Ratio Calculations for Mixed Occupancies:

$\frac{\text{Actual Area}}{\text{Allowable Area}} \leq 1$

c) Total Allowable Area for:
1) One Story: _____
2) Two Story: $A_a(2)$_____
3) Three Story: $A_a(3)$_____

d) Unlimited Area Building: Yes ✓ No _____ Code Section: 401

K. Fire Resistance Rating Requirements for Building Elements (hours).

Fire-Resistive Requirements:	Type: II-B
Bearing walls - Interior:	0
Exterior Non-Bearing Walls:	0
Structural Frame:	0
Partitions:	1
Shafts:	1 Hr.
Floors - Ceilings:	0
Roofs - Ceilings:	0

L. Design Occupant Load: EXIST. BLDG: 471 ADDITION: 120
Exit Width Required: 118" Exit Width Provided: 432"

M. Minimum Number of Required Plumbing Facilities:
a) Water Closets - Required (m) 2.5 (f) 5 Provided (m) 11 (f) 11
b) Lavatories - Required (m) 1.5 (f) 1.5 Provided (m) 6 (f) 6
c) Bath Tubs or Showers: _____
d) Drinking Fountains: _____ Service Sinks: _____

FOOTNOTES:

1) In case of conflict with the U.S. Department of Justice Federal Registers Parts I through V - ADA Guidelines and specific reference to the International Building Code Accessibility Chapters, the more restrictive requirement shall govern.

2) Additional Code Information shall be provided at the discretion of the Building Official for Complex Buildings. Including, but not limited to:
a) High Rise Requirements.
b) Atriums.
c) Performance Based Criteria.
d) Means or Egress Analysis.
e) Fire Assembly Locator Sheet.
f) Exterior and Interior Accessibility Route.
g) Fire Stopping, Including Tested Design Number.

PROJECT TEAM

OWNER:
DFCM
Kurt Baxter
Project Manager
4110 State Office Bldg.
Salt Lake City, Utah 84114
phone (801) 538-3174

USER:
UVSC
James L. Michaelis
Associate Vice President
Facilities Planning
800 West University Parkway
Orem, Utah 84058-5999
phone (801)863-8776

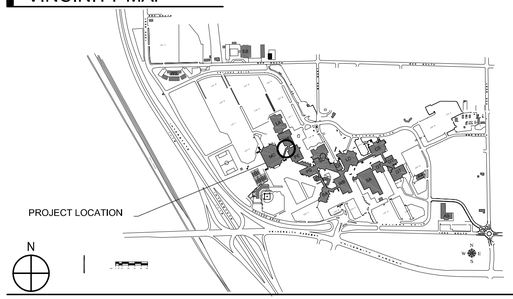
ARCHITECT:
AXIS ARCHITECTS
Pierre O. Langue AIA
610 I Street
Salt Lake City, Utah 84103
phone (801)355-3003
fax (801)355-8578

STRUCTURAL:
Bsumek Mu
Reinardt Bsumek
345 South 400 East
Salt Lake City, Utah 84111
phone (801)575-8223
fax (801)532-3778

MECHANICAL:
Spectrum Engineers
Wade Bennion
175 South Main Street
Salt Lake City, Utah 84111
phone (801)328-5151

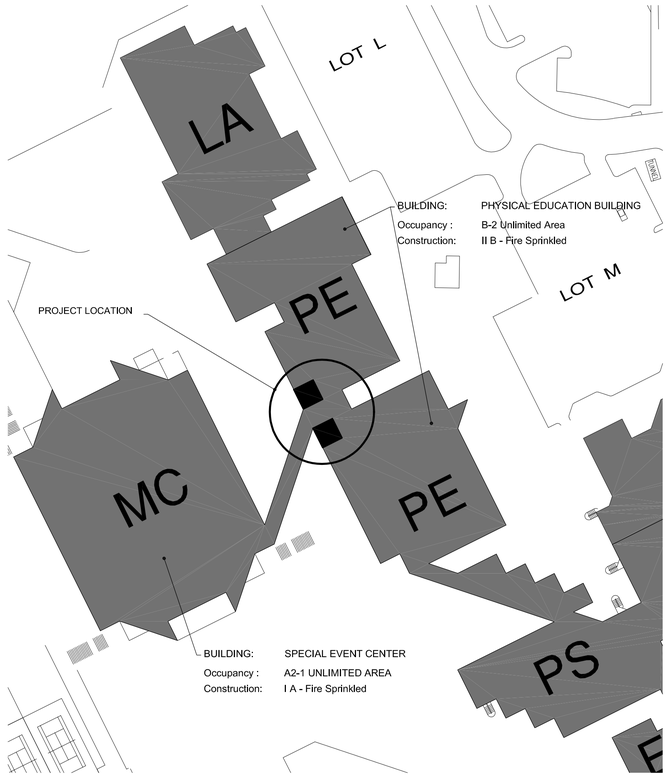
ELECTRICAL:
BNA Consulting
Carol Feldman
635 S. State Street
Salt Lake City, Utah 84111
phone (801)355-3003
fax (801)355-8578

VINCINITY MAP

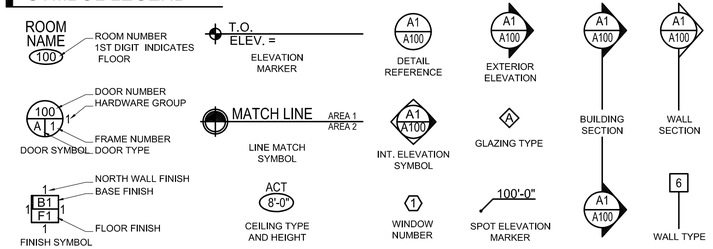


OVERALL SITE PLAN

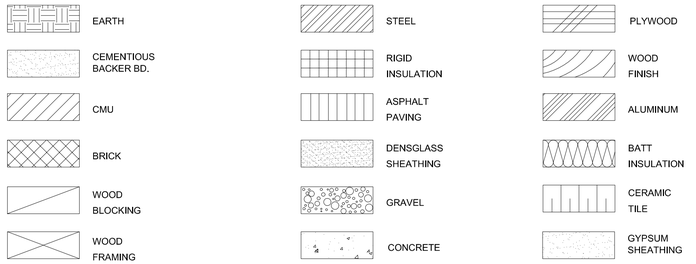
1"=100'-0"



SYMBOL LEGEND



GRAPHIC SYMBOLS



DRAWING INDEX

NOTE: THE SHEETS LISTED ABOVE REPRESENT A FULL SET OF CONSTRUCTION DOCUMENTS AND INCLUDE A DETAIL BOOK AND A PROJECT MANUAL AND SHALL NOT BE SEPERATED. ANY CONTRACTOR, SUBCONTRACTOR, VENDOR OR ANY OTHER PERSON PARTICIPATING IN OR BIDDING ON THIS PROJECT SHALL BE RESPONSIBLE FOR REVIEWING ALL THE CONSTRUCTION DOCUMENTS INCLUDING BUT NOT LIMITED TO DRAWINGS, DETAILS, PROJECT MANUAL, SPECIFICATIONS AND ANY AND ALL ADDENDA ISSUED.

UTAH VALLEY STATE COLLEGE
HALLWAY STUDY AREA INFILL
Orem, Utah

DFCM PROJECT # 05264790

Utah Valley State College
ENGINEERING, FACILITIES PLANNING & CONSTRUCTION DEPARTMENT

January 15 2006

State of Utah
Department of Administrative Services
Division of Facilities
Construction & Management
4110 State Office Building
Salt Lake City, Utah 84114
Phone: (801) 538 - 3018
Fax: (801) 538 - 3267

APPROVALS

Prime Agency _____ Date _____

DFCM _____ Date _____

APPROVAL DOES NOT RELIEVE A/E OF DESIGN LIABILITY

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INFORMATION SHEET
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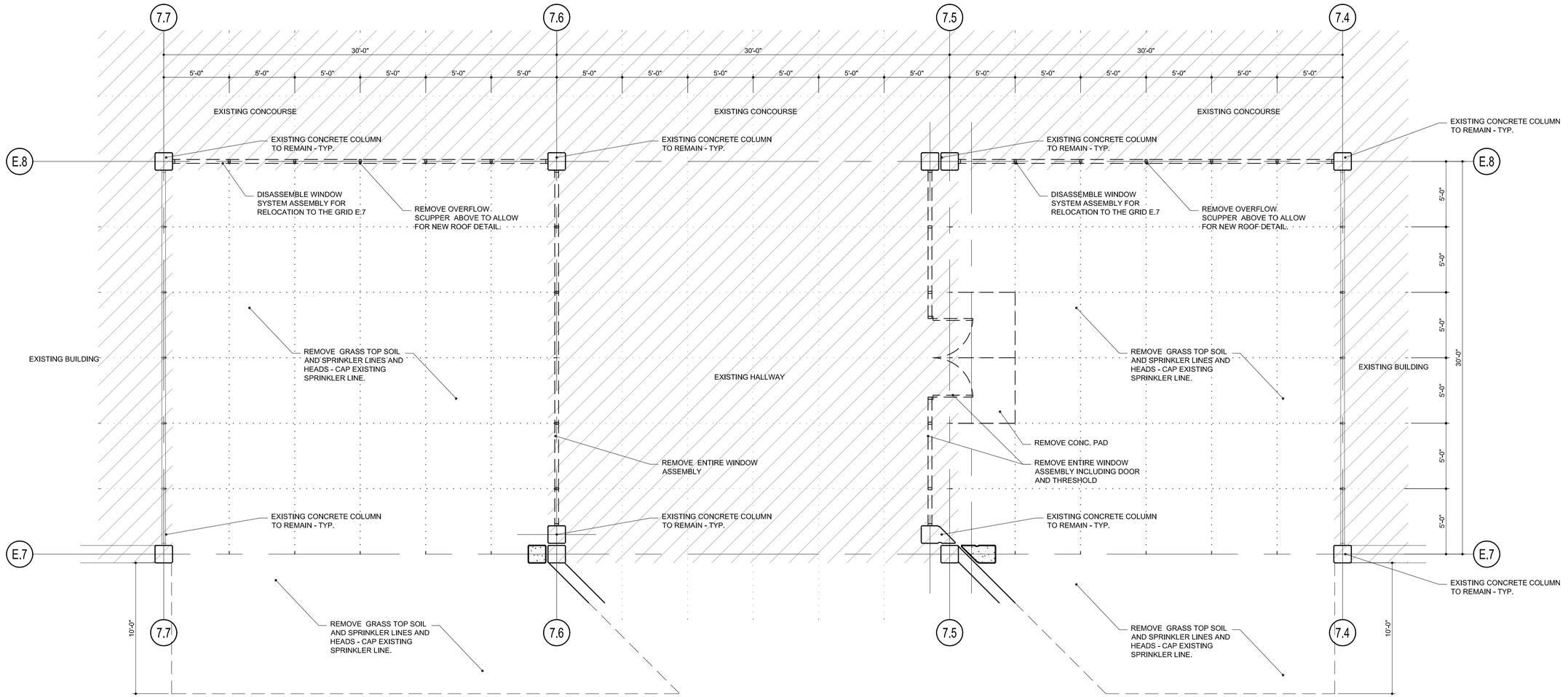
GI101

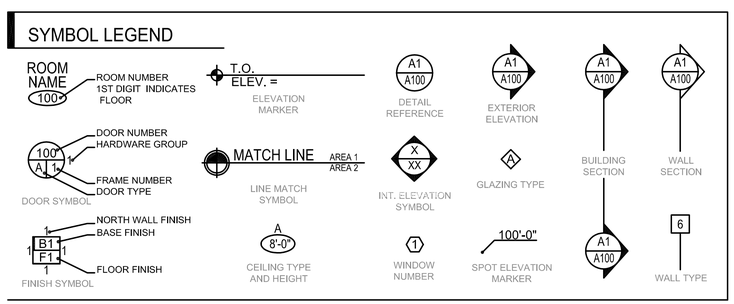


B3 DEMOLITION FLOORFLOOR PLAN

SCALE: 1/4"=1'-0"

PROJECT NORTH

SYMBOL LEGEND



NOTES

1. PROTECT EXISTING CONCRETE COLUMNS AND WALLS BEFORE POURING NEW CONCRETE AND DURING CONSTRUCTION.
2. PROTECT ALL EXISTING WORK DURING CONSTRUCTION.
3. CLEAN EXISTING CONCRETE COLUMNS
4. PROTECT EXISTING FINISHES INCLUDING CARPET. REPLACE CARPET WHERE DAMAGED DURING CONSTRUCTION.
5. REMOVE MOW STRIP AS REQUIRED FOR NEW CONSTRUCTION - REMOVE TO NEXT EXISTING JOINT - TYP.

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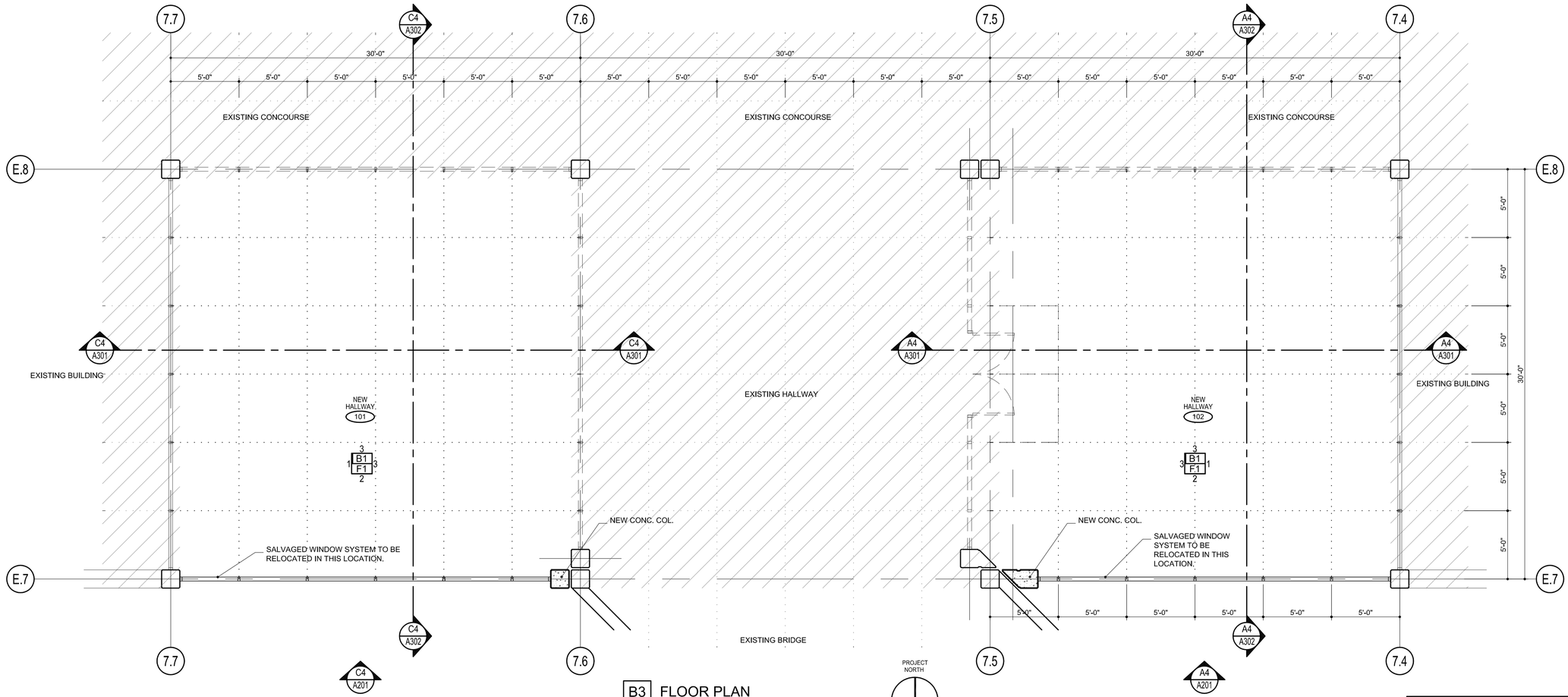
DEMOLITION PLAN

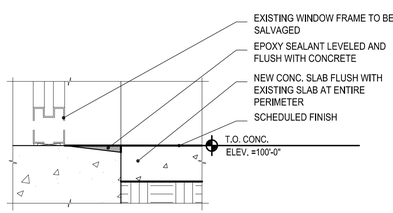
AD101



B3 FLOOR PLAN

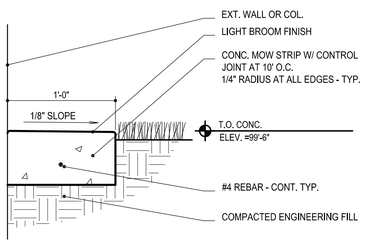
SCALE: 1/4"=1'-0"

PROJECT NORTH



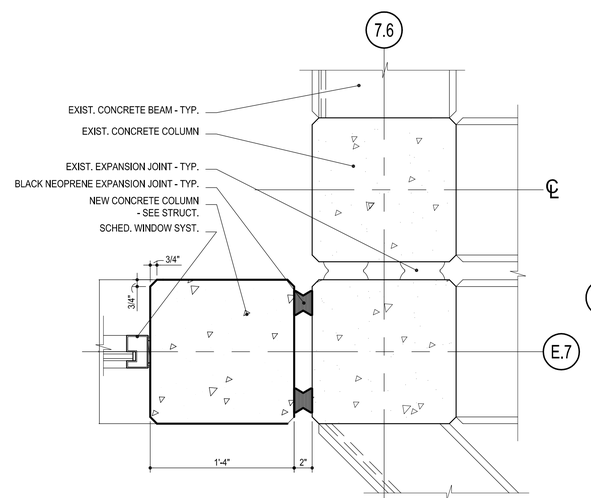
B1 NEW/ OLD CONC. TRANSITION DETAIL

SCALE: 1 1/2"=1'-0"



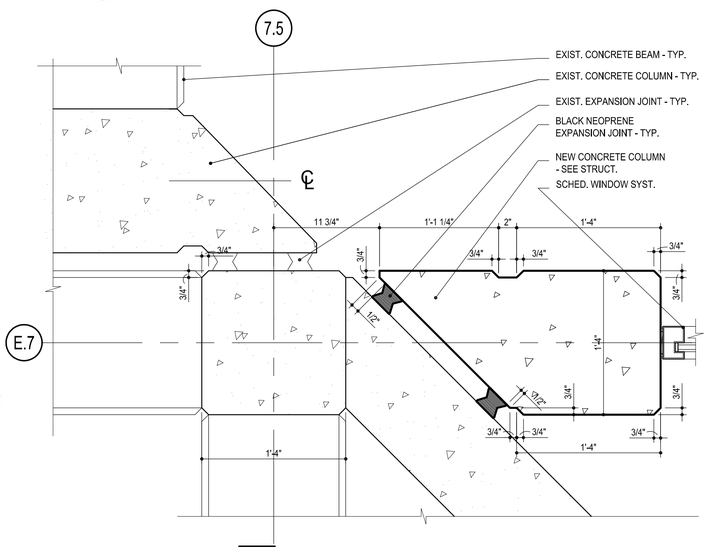
A1 MOW STRIP DETAIL

SCALE: 1 1/2"=1'-0"



A2 COLUMN DETAIL

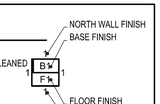
SCALE: 1 1/2"=1'-0"



A4 COLUMN DETAIL

SCALE: 1 1/2"=1'-0"

FINISH LEGEND



WALL:
1 EXISTING GLASS SYSTEM TO BE CLEANED
2 NEW RELOCATED GLASS SYSTEM
3 OPEN

FLOOR: F1 CARPET

BASE: B1 NO BASE

1. MATERIALS IN SIMILAR LOCATIONS ARE NOT KEYNOTED AT EVERY OCCURRENCE.
2. PROVIDE CONCRETE MOISTURE SEALER ALL AREAS TO RECEIVE VCT, CAPRET, SHEET VINYL, WALK-OFF MAT AND/OR RUBBER FLOORING.

NOTES

1. CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION

2. CONTRACTOR TO INCLUDE IN THEIR BID THE POTENTIAL COST OF GLASS BREAKING DURING DISASSEMBLY, STORAGE, REASSEMBLY, ETC. .

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FLOOR PLAN

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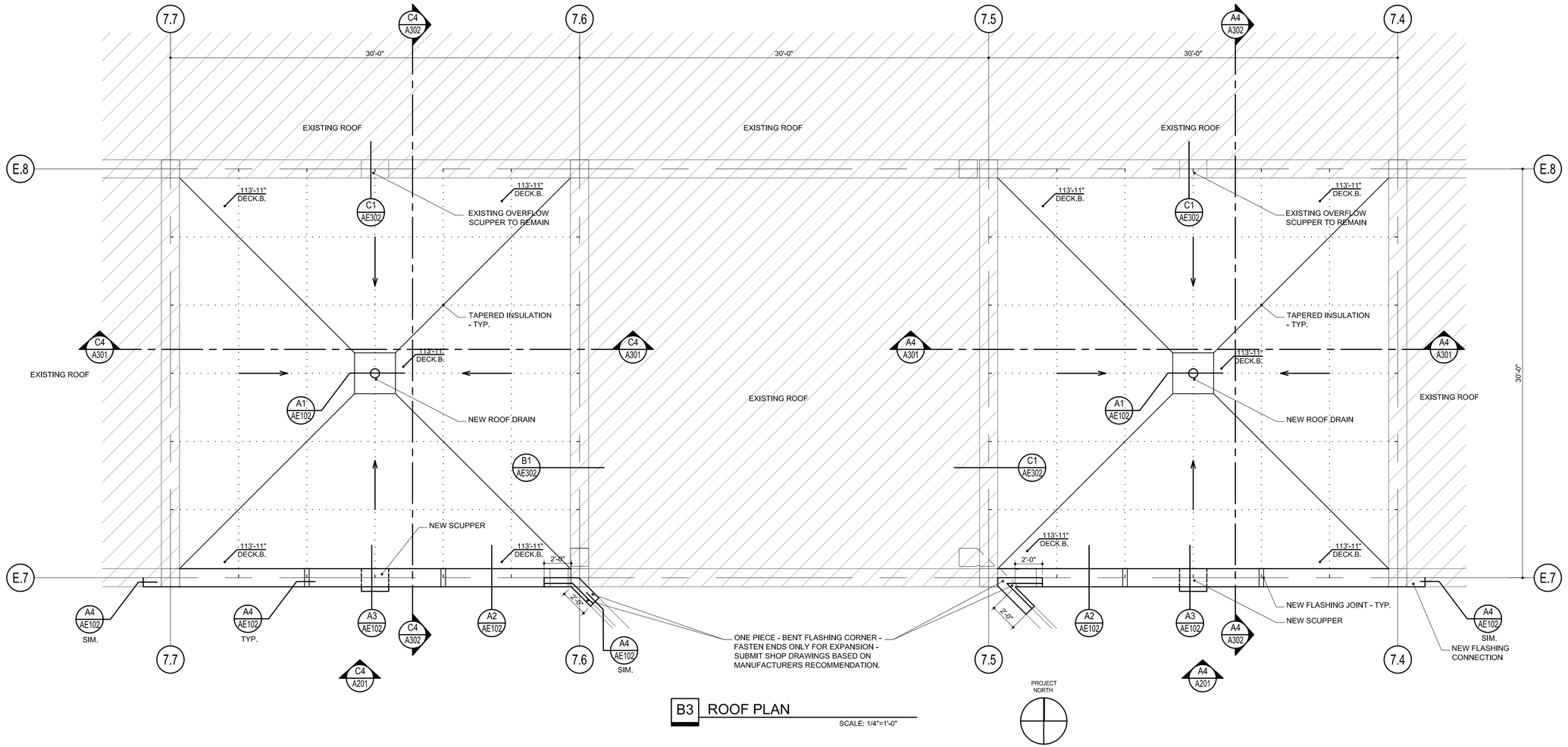
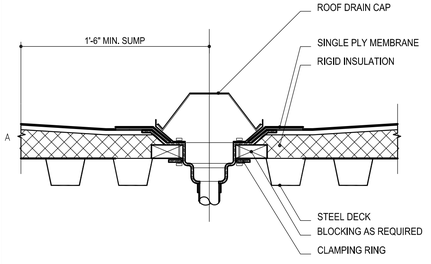


B3 ROOF PLAN

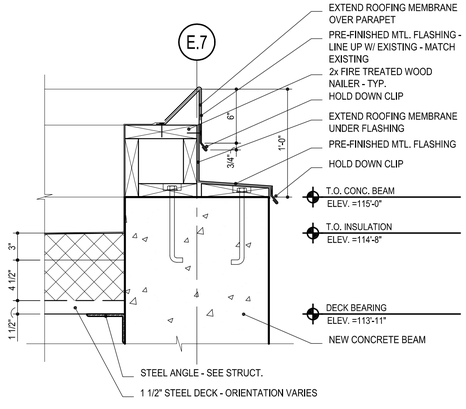
SCALE: 1/4"=1'-0"

PROJECT NORTH



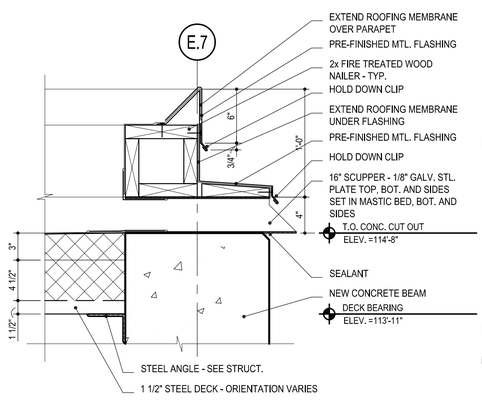
A1 ROOF DRAIN DETAIL

SCALE: 1 1/2"=1'-0"



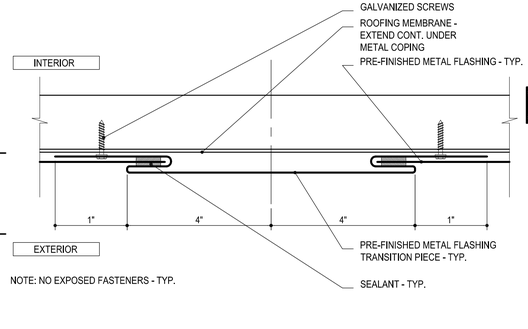
A2 COPING DETAIL

SCALE: 1 1/2"=1'-0"



A3 SCUPPER DETAIL

SCALE: 1 1/2"=1'-0"



NOTE: NO EXPOSED FASTENERS - TYP.

A4 FLASHING JOINT DETAIL

SCALE: FULL SCALE

NOTES

1. PROVIDE TAPERED INSULATION TO CREATE SLOPE.
2. MATCH NEW INSULATION ELEVATION WITH EXISTING AT EXISTING SCUPPERS.
3. REPAIR EXISTING ROOFING MEMBRANE 5' PAST PARAPETS TYP.

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UVSC HALLWAY ADDITION - DFCM

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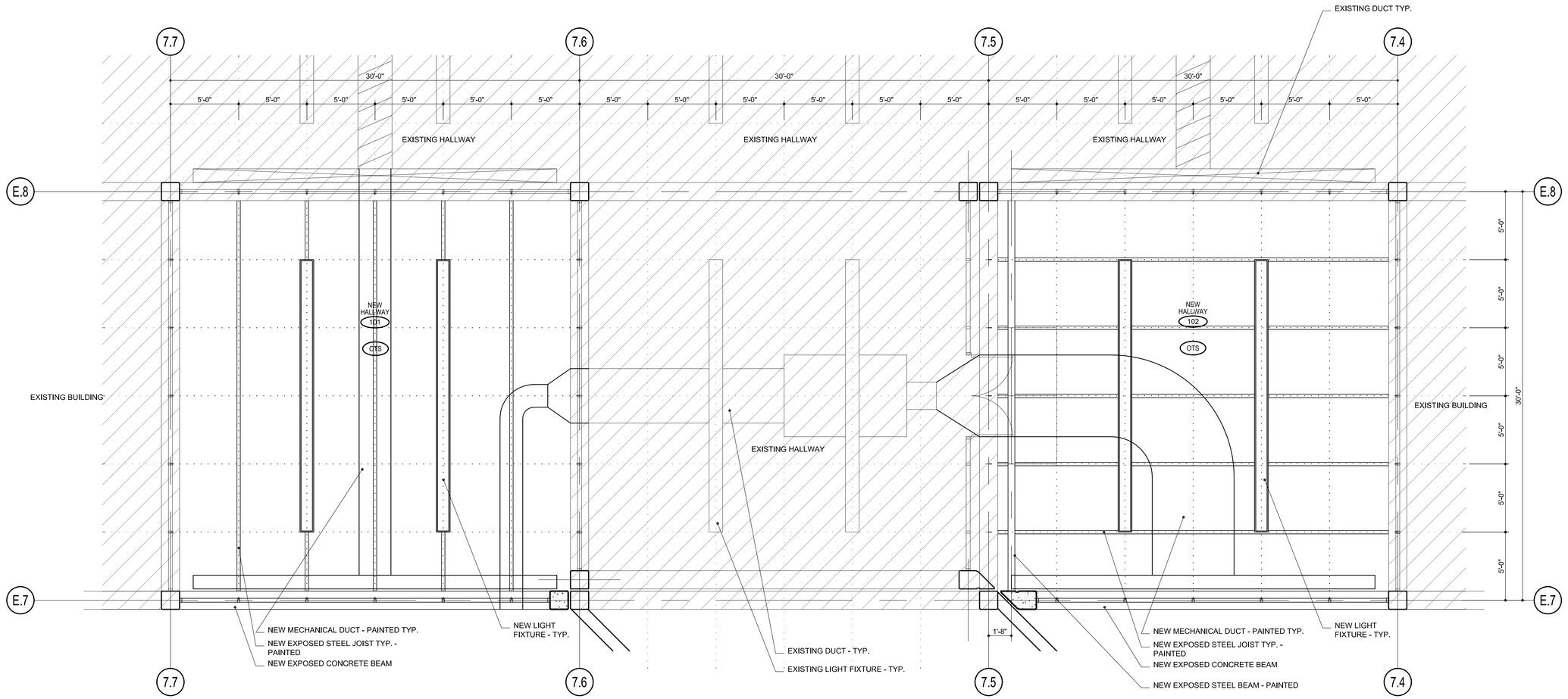
ROOF PLAN

AE102



B3 REFLECTED CEILING PLAN

SCALE: 1/4"=1'-0"

PROJECT NORTH

CEILING SYMBOL LEGEND

- FLUORESCENT LIGHT FIXTURE, REFER TO ELECTRICAL DRAWINGS
- RETURN AIR REGISTER, REFER TO ELECTRICAL DRAWINGS.
- SUPPLY AIR DIFFUSER, REFER TO ELECTRICAL DRAWINGS.
- (S) SMOKE DETECTOR

ALSO REFER TO ELECTRICAL AND MECHANICAL DRAWINGS

CEILING SCHEDULE

A — MATERIAL
8'-0" — ELEVATION ABOVE F.F.

(OTS) OPEN TO STRUCTURE, PAINTED INCLUDES ALL EXPOSED ITEMS INCLUDING DUCTS, PIPES, STEEL CONNECTIONS, BEAMS JOISTS

NOTES

1 ALL MECHANICAL DUCTS, PIPES, ELECTRICAL CONDUIT AND FIRE SPRINKLER LINES TO BE PAINTED WITH CAMPUS COLOR CODED SYSTEM.
2 ALL MECHANICAL DUCTS, PIPES, ELECTRICAL CONDUIT AND FIRE SPRINKLER LINES ARE SHOWN FOR COORDINATION PURPOSE ONLY.
3 ALL EXPOSED STRUCTURAL STEEL TO BE PAINTED TO MATCH EXISTING, INCLUDING MTL DECK, BEARING PLATES, ANGLES, JOIST BRACING ETC.

4 UVSC STANDARD COLORS ARE:
Java Green: HVAC Duct AC098N
Sunburry Low hide: cold water supply AC105Y
El sol low hide: cold water return AC 106Y
serengeti sun low hide: Hot water supply AC108Y
Fire King Red low hide: Hot water return AC 110Y
Lime Burst: Domestic water supply and return 7205A
Blue Brilliance: Control compressed air AC 079N
Sizzling Haute: Electrical Supply & Sprinkler line AC119R

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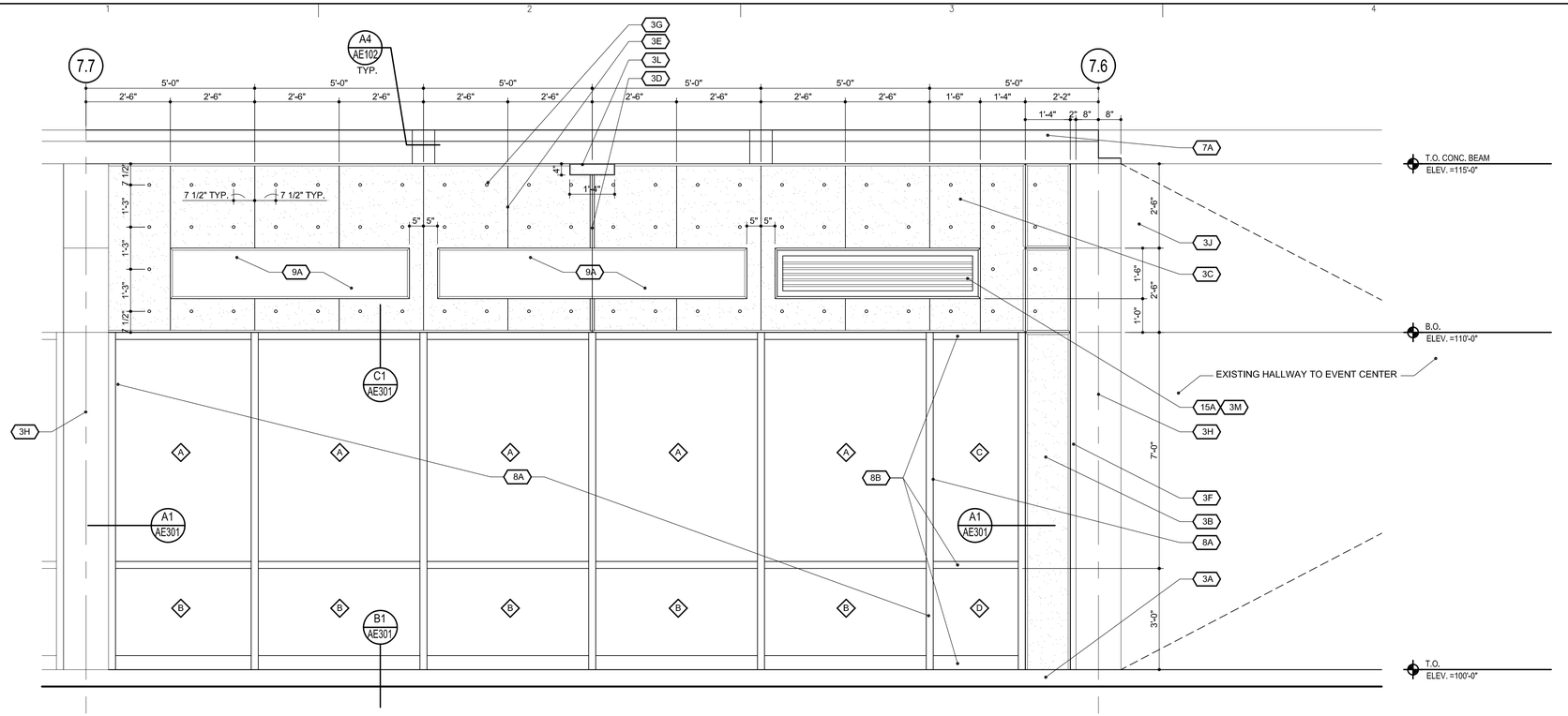
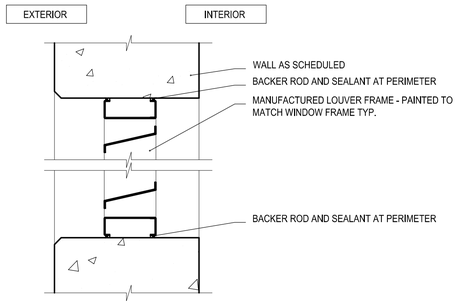
REFLECTED CEILING PLAN

AE111



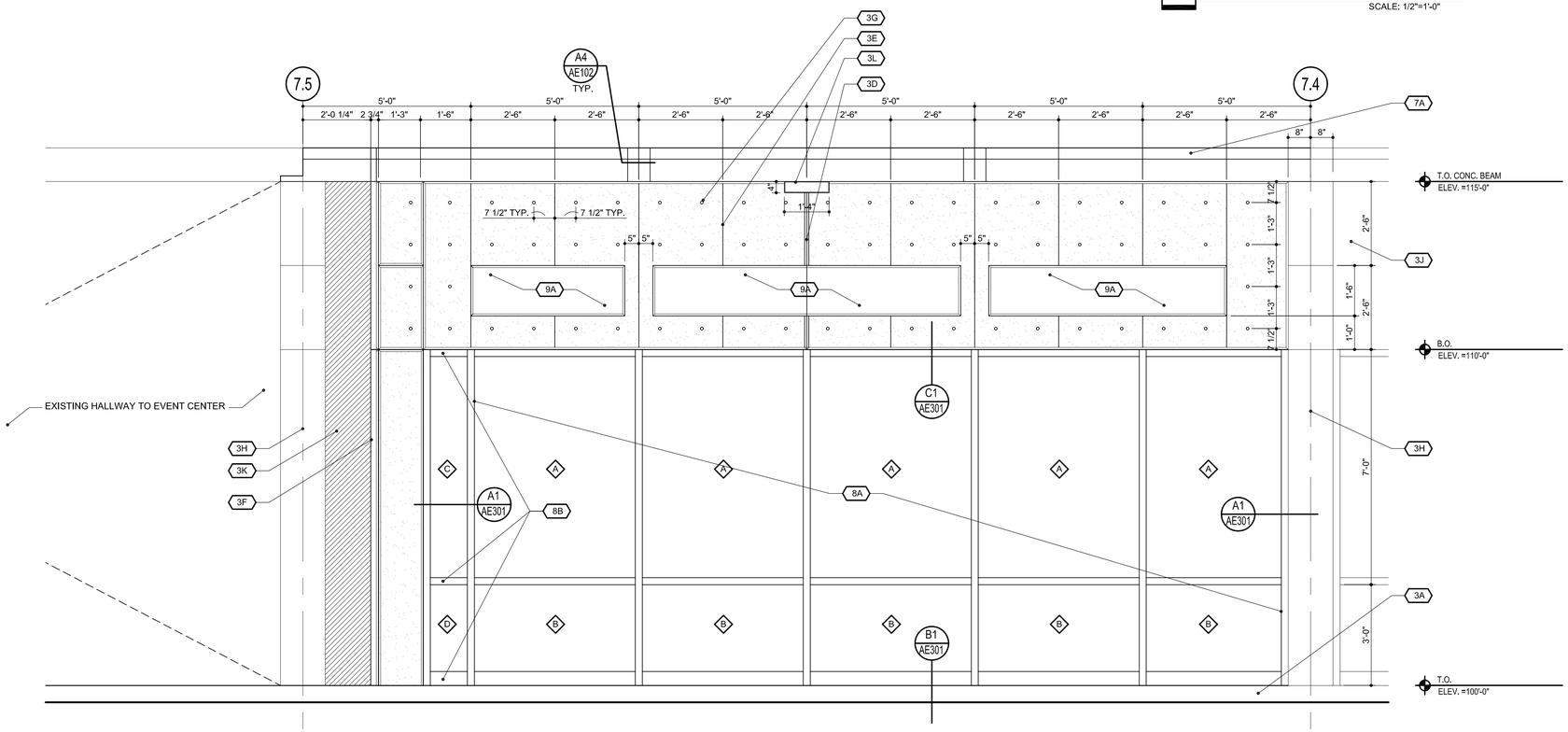
C4 ENLARGED PARTIAL WEST ELEVATION

SCALE: 1/2"=1'-0"



D5 LOUVER DETAIL

SCALE: 1 1/2"=1'-0"



A4 ENLARGED PARTIAL WEST ELEVATION

SCALE: 1/2"=1'-0"

GLAZING SCHEDULE

- A — EXISTING 1" INSULATED GLASS - BRONZE TINT
- B — EXISTING 1" INSULATED TEMPERED GLASS - BRONZE TINT
- C — NEW 1" INSULATED GLASS - BRONZE TINT
- D — NEW 1" INSULATED TEMPERED GLASS - BRONZE TINT

1. CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS AND ANGLES PRIOR TO FABRICATION.
2. SOME DETAILS MAY BE REVERSED.

KEYNOTE LEGEND

1A — ELEVATIONS

2 - SITE WORK:

- 2A — SLOPE GRADE AWAY FROM BLDG. TYP.

3 - CONCRETE:

- 3A — NEW CONCRETE FOUNDATION WALL - SEE STRUCT.
- 3B — NEW CONCRETE COLUMN - SEE STRUCT.
- 3C — NEW CONCRETE BEAM - SEE STRUCT.
- 3D — CONTROL JOINT
- 3E — FORM JOINT
- 3F — 2" EXPANSION JOINT
- 3G — CONCRETE FORM TIE
- 3H — EXISTING CONC. COLUMN TO REMAIN
- 3J — EXISTING CONC. BEAM TO REMAIN
- 3K — EXISTING CONC. WALL TO REMAIN
- 3L — CUT-OUT IN CONC. BEAM WITH GALV. STEEL SCUPPER
- 3M — OPENING IN CONC. BEAM FOR MECHANICAL LOUVER

7 - THERMAL AND MOISTURE PROTECTION:

- 7A — PRE-FINISHED METAL FLASHING

8 - DOORS AND WINDOWS:

- 8A — EXISTING WINDOW SYSTEM - RELOCATED
- 8B — NEW MULION TO MATCH EXISTING OR CUT EXISTING MULION.

9 - FINISHES

- 9A — RECESS PAINTED DARK BRONZE - SMOOTH CONC. DO NOT SANDBLAST

15 - MECHANICAL

- 15A — LOUVER SEE MECHANICAL

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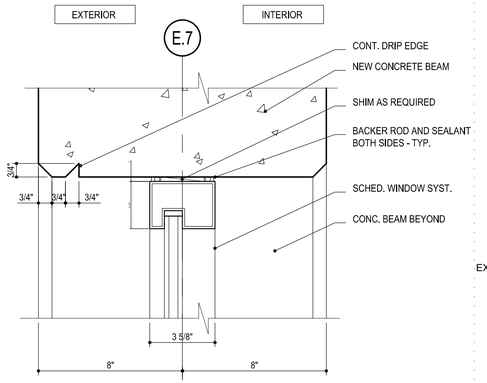
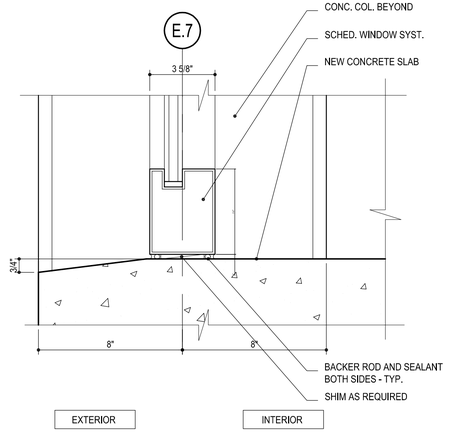
ELEVATIONS

AE201



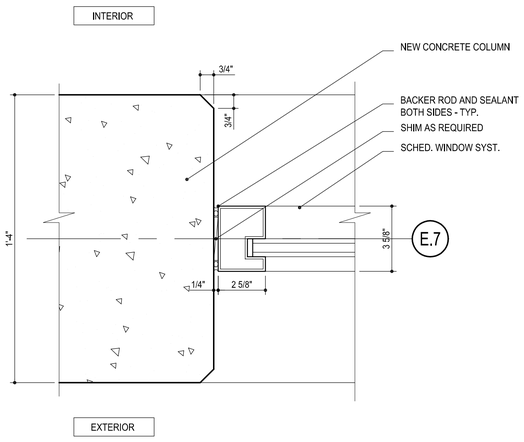
C1 HEAD DETAIL

SCALE: 3"=1'-0"



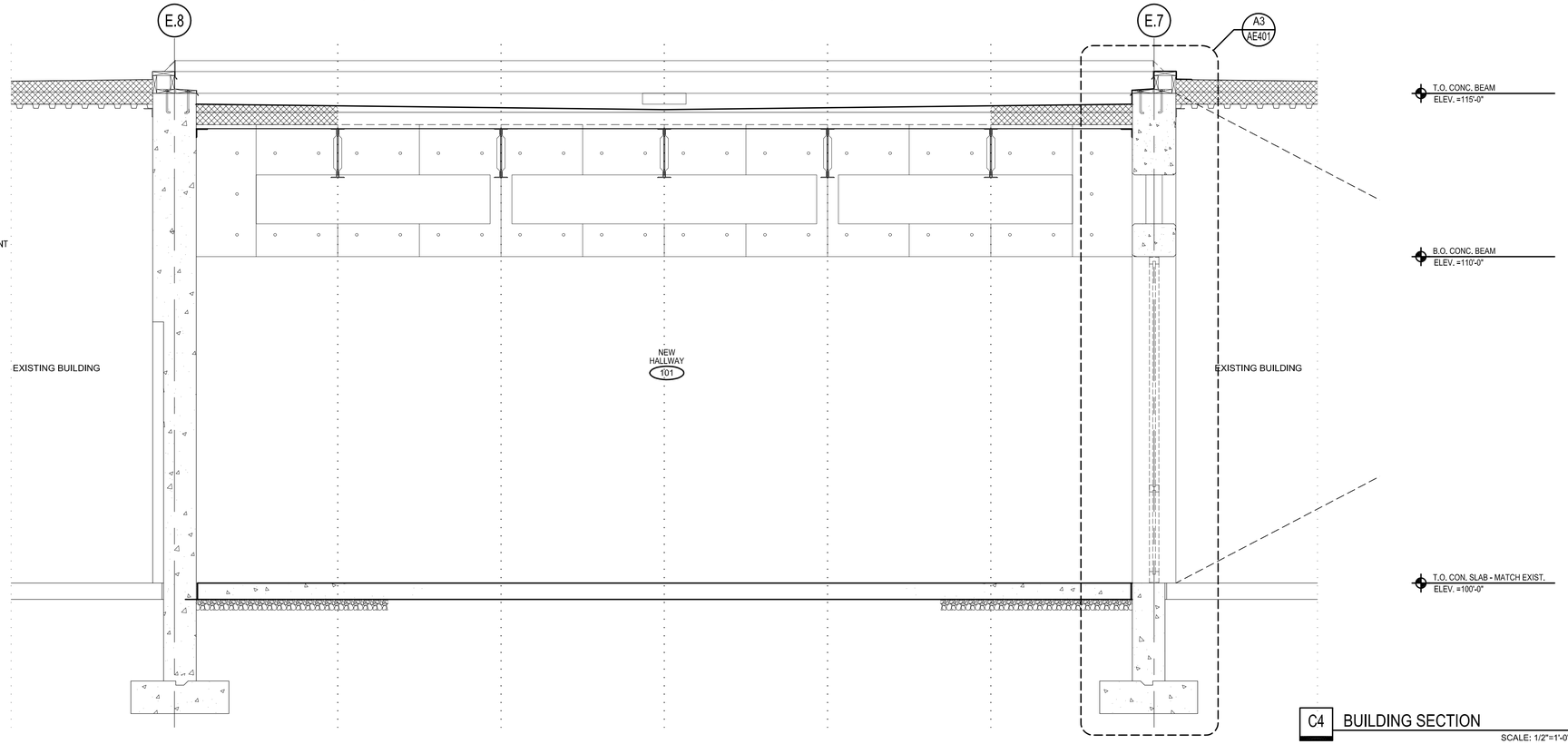
B1 SILL DETAIL

SCALE: 3"=1'-0"



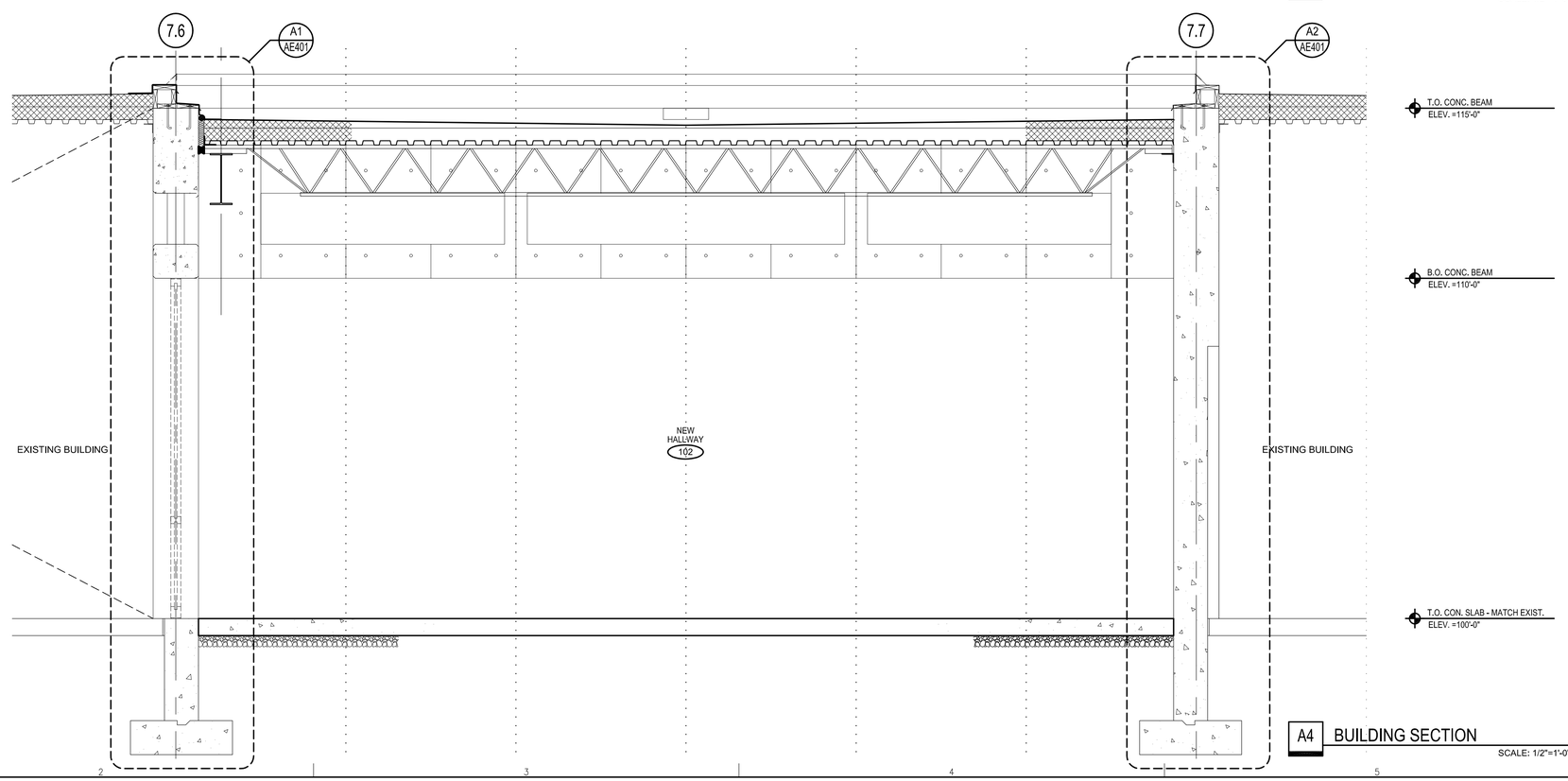
A1 JAMB DETAIL

SCALE: 3"=1'-0"



C4 BUILDING SECTION

SCALE: 1/2"=1'-0"



A4 BUILDING SECTION

SCALE: 1/2"=1'-0"

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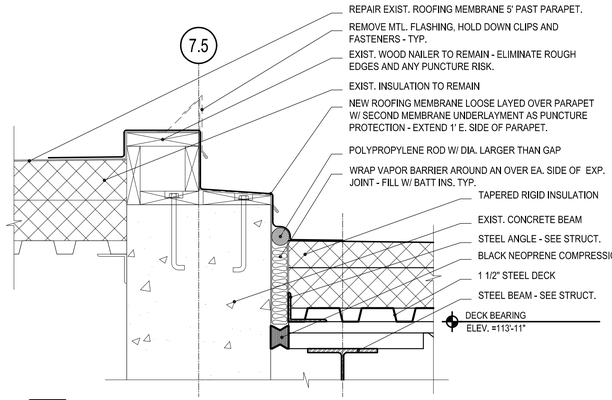
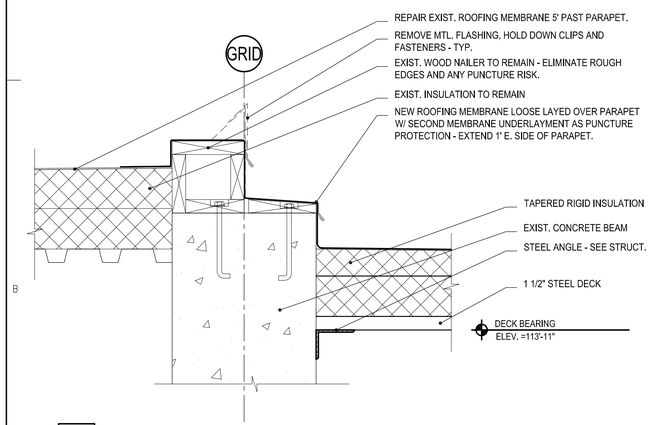
BUILDING SECTIONS

AE301



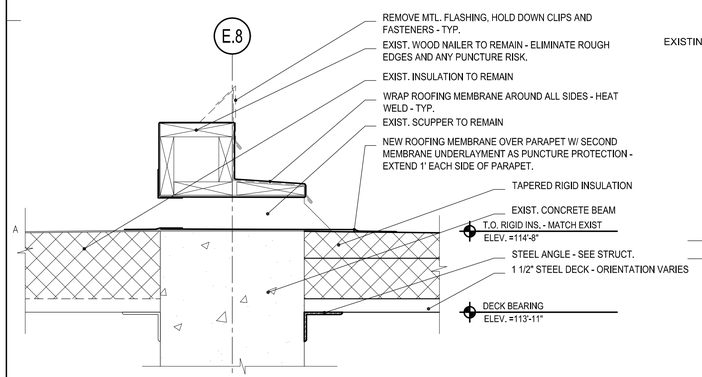
C1 ROOF DETAIL AT EXPANSION JOINT

SCALE: 1 1/2"=1'-0"



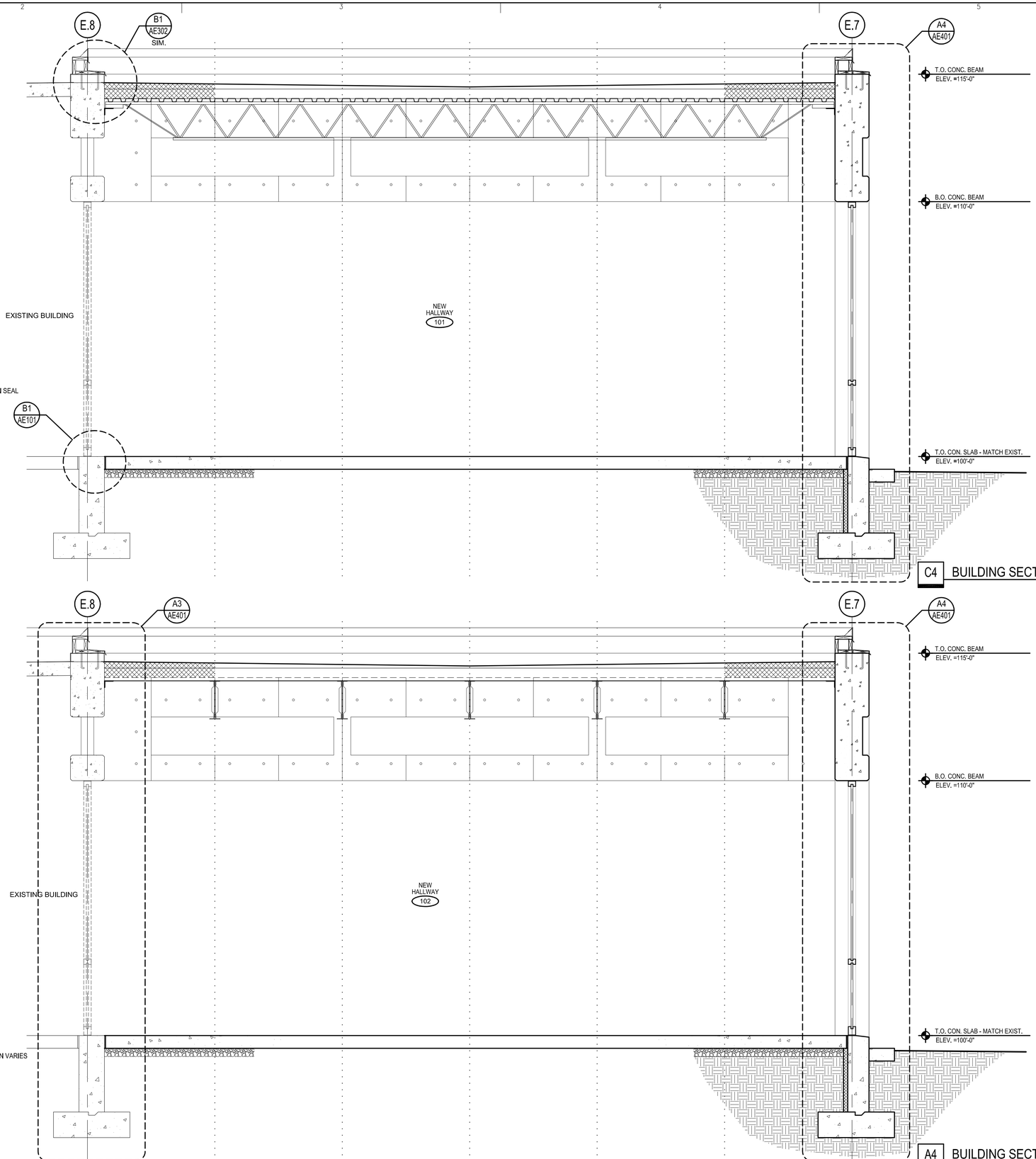
B1 ROOF DETAIL AT EXIST. PARAPET

SCALE: 1 1/2"=1'-0"



A1 ROOF DETAIL AT EXIST. SCUPPER

SCALE: 1 1/2"=1'-0"



C4 BUILDING SECTION

SCALE: 1/2"=1'-0"

A4 BUILDING SECTION

SCALE: 1/2"=1'-0"

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DFCM

UVSC HALLWAY ADDITION - DFCM

PHYSICAL EDUCATION BLDG UVSC CAMPUS

Revision #	Date
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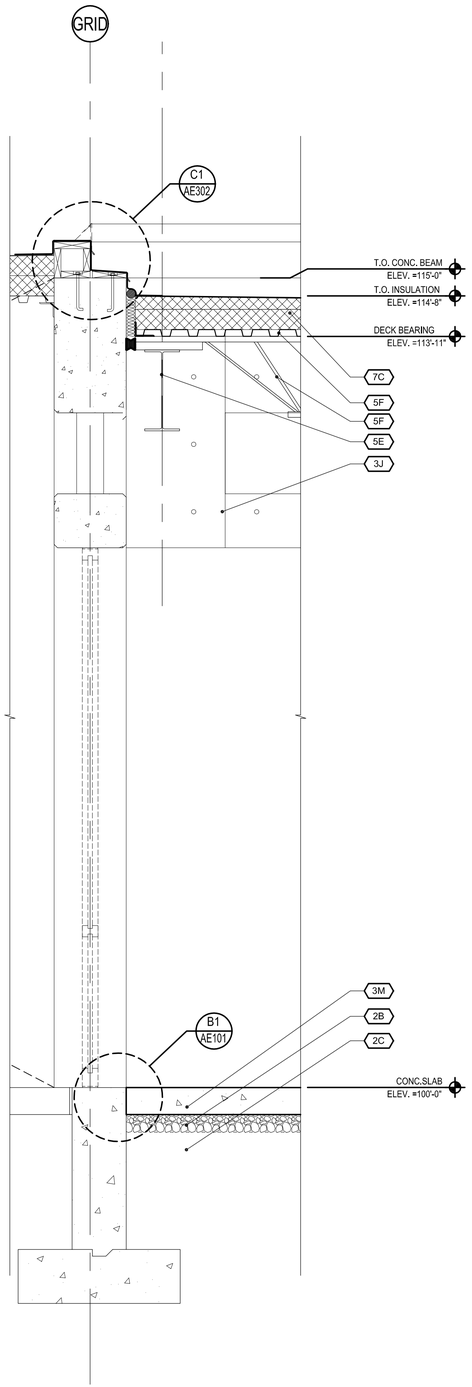
Axis Job #	0510
Owner #	DFCM # 05264790
Date	01-15-06
Drawn	-
Checked	-

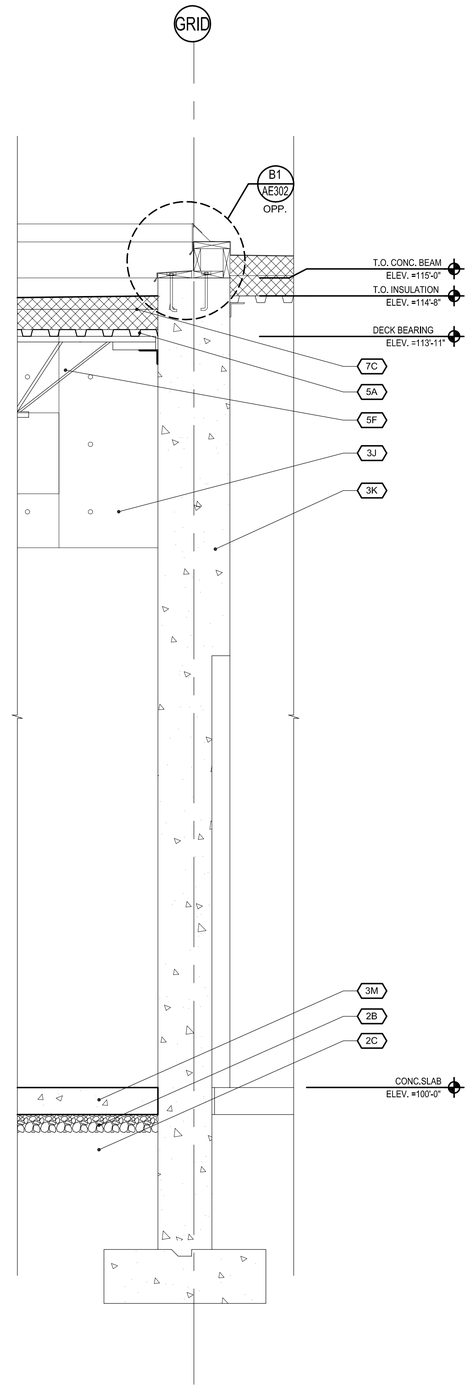
BUILDING SECTIONS

AE302



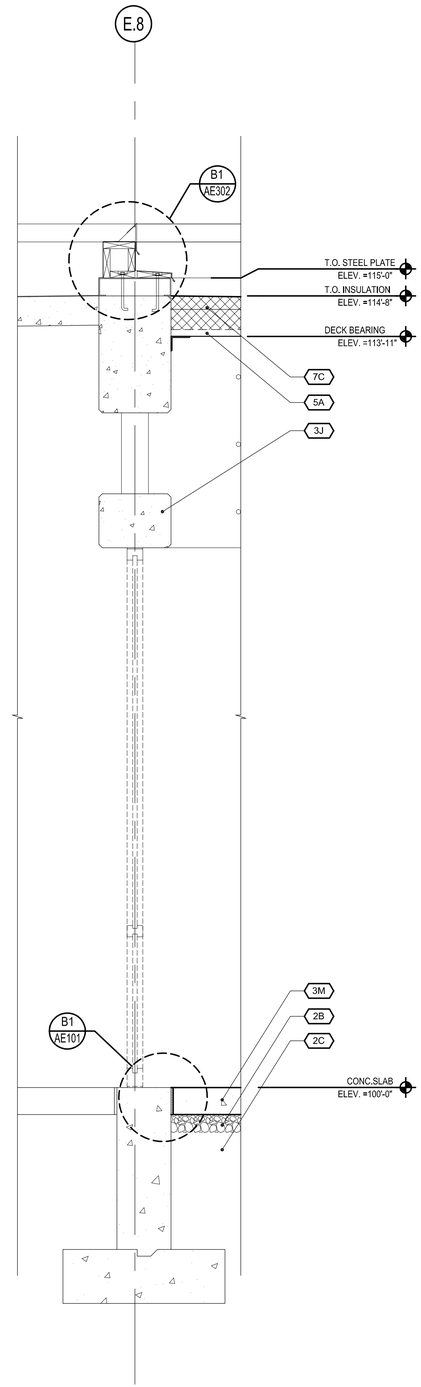
A1 WALL SECTION

SCALE: 3/4"=1'-0"



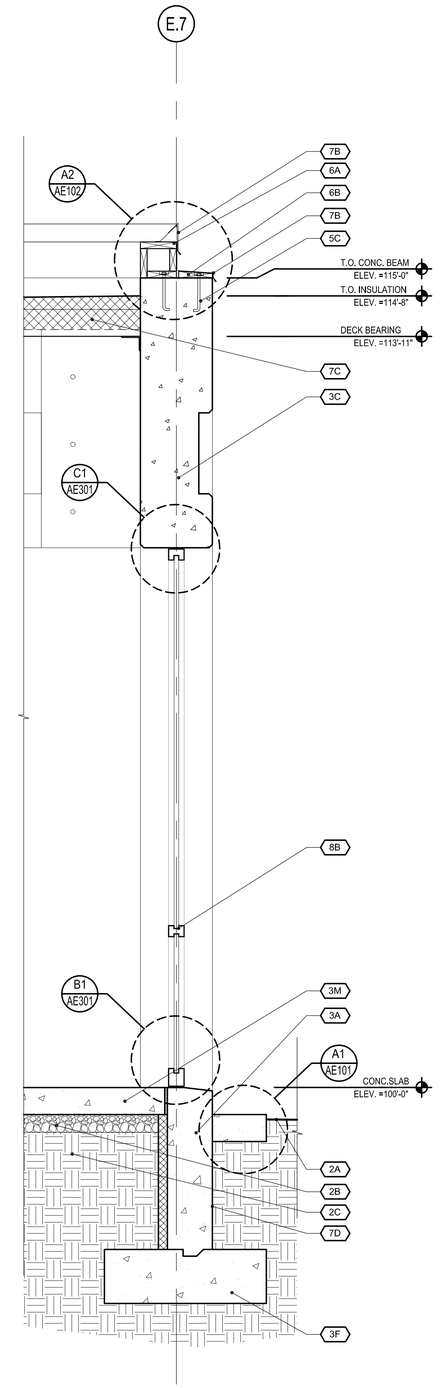
A2 WALL SECTION

SCALE: 3/4"=1'-0"



A3 WALL SECTION

SCALE: 3/4"=1'-0"



A4 WALL SECTION

SCALE: 3/4"=1'-0"

KEYNOTE LEGEND 1A

WALL SECTIONS

2 - SITE WORK:
- 2A SLOPE GRADE AWAY FROM BLDG. TYP.
- 2B 4" GRAVEL BASE
- 2C ENGINEERING FILL

3 - CONCRETE:
- 3A NEW CONCRETE FOUNDATION WALL - SEE STRUCT.
- 3B NEW CONCRETE COLUMN - SEE STRUCT.
- 3C NEW CONCRETE BEAM - SEE STRUCT.
- 3D CONTROL JOINT
- 3E FORM JOINT
- 3F NEW CONCRETE FOUNDATION WALL - SEE STRUCT.
- 3G CONCRETE FORM TIE
- 3H EXISTING CONC. COLUMN TO REMAIN
- 3J EXISTING CONC. BEAM TO REMAIN
- 3K EXISTING CONC. WALL TO REMAIN
- 3L RECESS IN CONC. BEAM W/ GALV. STEEL SCUPPER
- 3M NEW CONCRETE SLAB - SEE STRUCT.

5 - METALS:
- 5A 1 1/2" STEEL DECK - SEE STRUCT.
- 5B STEEL ANGLE - PAINTED - SEE STRUCT.
- 5C 3/8" DIA. ANCHOR BOLT AT 32" O.C.
- 5D 1/8" BENT GALV. STEEL PLATE. ALIGN 1/8" VERT. BUTT JOINTS - CAULK AND PAINT - TYP.
- 5E STEEL BEAM - PAINTED SEE STRUCT.
- 5F STEEL JOIST - PAINTED SEE STRUCT.

6 - WOOD:
- 6A 2x TREATED WOOD BLOCKING
- 6B SHAPED 2x TREATED WOOD BLOCKING

7 - THERMAL AND MOISTURE PROTECTION:
- 7A SINGLE PLY ROOFING MEMBRANE WITH GRAVEL BALLAST
- 7B CONT. PRE-FINISHED METAL FLASHING
- 7C RIGID INSULATION - SEE ROOF DETAILS
- 7D WATERPROOFING
- 7E 2" EXPANSION JOINT

8 - DOORS AND WINDOWS:
- 8A SCHEDULED DOOR AND FRAME
- 8B SCHEDULED WINDOW SYSTEM

9 - FINISHES:
- 9A PAINTED G. B.
- 9B SCHEDULED FLOOR FINISH
- 9D SUSPENDED PAINTED G. B. CEILING

15 - MECHANICAL:
- 15A MECH. LOUVER - PAINTED WITH UVSC COLORS.
- 15B MECH. LOUVER - PAINTED

16 - ELECTRICAL:
- 16A LIGHT FIXTURE - SEE SCHEDULE

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Revision #	Date
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Axis Job #	0510
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Drawn	PL
Checked	PL

WALL SECTIONS

AE401

I. Basis of Design

B1. Governing Building Code: 2003 International Building Code (IBC).

B2. Gravity Loading
A. Roof.....20 psf dead load
Snow Loads Plus Snowdrift
Pg= 43 psf
Pf= .33 psf
Ce= 1.0
Is= 1.0
Ct= 1.0

B3. Earthquake
A. Seismic Use Group.. II, IE = 1.0
B. SDS = .8, SD1 = .484,
C. Site Class = D
D. Lateral Force Resisting System = Dual System Special Concrete Moment Frame and Concrete Shear Wall
E. Base shear = .1W R= 8
F. Analysis procedure = Equivalent Lateral-Force Procedure
G. Seismic design category = D

B4. Wind velocity.....90 miles per hour (3 second gust speed) exposure C.
Net uplift force at roof equals 17 psf.
Importance Factor Iw.....1.15

B5. Foundation
A. Soil Bearing Pressure.....3000 psf as per original construction document November 1993.

II. Site Work

SW1. A minimum of 12" of topsoil shall be removed from the entire building site including all vegetation and debris.

SW2. The Contractor shall retain a Soils Engineer to verify 3000 psf soil pressure prior to placing any footing forms. Report to Architect any findings which may affect foundation and footing stability.

SW3. All bearing earth to be undisturbed earth or compacted fill. The area on which the fill is placed must be frost free. The fill shall then be placed in layers not to exceed 8 inches in depth and compacted.

SW4. All fill and back fill shall be compacted to a minimum of 95% of maximum relative density for footings and 90% for all other structural fill based on AASHTO T180.

SW5. Any fill to be placed under the building and footings shall be a well graded granular material within the limits of the following gradation, unless otherwise specified by Soils Engineer:

Sieve Size	Percent Passing
4"	100
1"	60–90
#4	30–60
#40	10–30
#200	5–12

SW6. All water shall be removed from foundation excavation prior to placing of concrete. Do not pour concrete under water.

SW7. Any unusual soil conditions (water, clay, soft layers, etc.) encountered during excavation for footings shall be immediately brought to the attention of the architect and soils engineer.

SW8. All foundation excavations shall be protected from all detrimental changes in environmental conditions such as rain and freezing.

SW9. Contractor shall coordinate the architectural, structural, and civil drawings for top of footing elevations and footing steps and excavations.

SW10. Contractor shall verify all existing and future grades. Footings shall be poured to maintain the minimum frost protection or confinement indicated on details. Footing steps shall be provided as per typical details where required by site conditions.

SW11. See geotechnical report for subgrade preparation typical.

III. Concrete and Reinforcing

C1. All work and materials shall comply with all areas of ACI.318 and ACI 347 Publications and applicable ASTM Publications.

C2. Compressive strength of concrete at 28 days shall be as follows: (only 1–grade of concrete shall be poured on the job at one time). Use type II cement in contact with ground.

	Minimum Compressive Strength(psi) (At 28 Days)	% Slump (+/– 1/2")	Maximum Air Entrainment	Aggregate Size	Inspection Required
Footings	3000	4	NO	1 1/2"	NO
Foundations	4000	4	NO	3/4"	NO
Int.Slab on Grade	4000	3	NO	1 1/2"	NO
Ext.Slab on Grade	4000	3 5 1/2% to 7 1/2%		1 1/2"	NO
Other	4000	4	NO	3/4"	YES

C3. Hardrock aggregates shall conform to ASTM C–33. Their Maximum size shall be 3/4" except 1–1/2" shall be used for footings and slabs on grade.

C4. Admixtures
A. Concrete mix shall include flyash as per ASTM C618 class "F" except that maximum loss on ignition shall be limited to 1% to yield specified quantities. Flyash replacement of cement shall be limited to 20% by weight.

C5. The contractor shall submit mix design and 3, 7, and 28 days strength tests for review by the structural engineer before any concrete is poured at the job site.

C6. All concrete that is placed by pumping shall be medium range–plasticized with water reducing admixture which shall comply with specifications for chemical admixtures for concrete, ASTM designation C–494 non–chloride and shall be used in strict accordance with manufacturer's recommendations. Product specification publication shall be submitted to structural engineer for review.

C7. Unless otherwise noted all reinforcement bars shall be securely anchored to the forms and spaced from them as follows: Minimum Coverage
A. Cast against & exposed to earth.....3 inches
B. Concrete exposed to earth or weather:
#6 though #18 bars.....2 inches
#5 bar and smaller.....1 1/2 inches
C. Not exposed to weather or in contact with ground: slabs, walls, joists:.....3/4 inches

C8. Reinforcing Steel
A. All reinforcing steel shall be bent, detailed and chaired as per the "ACI Manual of Standard Practice for Detailing Reinforcing Concrete Structures."
B. All reinforcing steel to be welded shall comply with ASTM A706.
C. All reinforcing steel shall be of new stock deformed bars conforming to ASTM A–615 grade 60 unless otherwise noted. Placement of bars in accordance with ACI 315 and ACI 318. Use bar supports per ACI 315 chapter 7 for all rebar and welded wire fabric. As per ACI 318, Section 7.5.1: "All reinforcement shall be accurately placed and adequately supported before concrete is placed and shall be secured against displacement within tolleranced permitted in 7.5.2." Wet stabbing reinforcing is not allowed.
D. Unless otherwise indicated, all anchors welded to steel plates or angles that are embedded in masonry or concrete shall be deformed bar anchors conforming to A36 Steel or ASTM A706.
E. Minimum lap lengths shall be, unless noted otherwise on drawings: #3=18" #4=19" #5=24" #6=29" #7=42" #8=48" #9=54" #10=58" #11=65".
Epoxy coated bar laps, see note M.
Leightweight concrete bar laps, multiply above values by 1.3.
See shear wall schedule for seismic lap lengths.
All vertical reinforcing bars (unless noted otherwise) shall be doweled to footing with 90 degree standard hook.
F. Reinforcing for concrete walls as follows:(unless otherwise noted on drawings)

Thickness	Horizontal Reinf	Vertical Reinf
6"	#4 at 16" o.c. each face	#4 at 18" o.c. each face
8"	#4 at 12" o.c. each face	#4 at 18" o.c. each face
10"	#4 at 15" o.c. each face	#4 at 18" o.c. each face
12"	#4 at 10" o.c. each face	#4 at 16" o.c. each face

G. All dowels shall have at least 38 bar diameter embedment.
H. Break out dowels may be used where required by contractor, however dowels shall be Grade 40 and spacing of dowels shall be decreased by 1/3.
I. Provide corner bars at all intersecting corners. Use same size bar and spacing as horizontal wall reinforcing.
J. Add 2–#5 bars around all openings (unless otherwise noted) and extend 24" beyond corner of openings. Add also 2–#5 x 4'–0 diagonally at corners. For reinforcing over opening see opening details on drawings.
K. When called for on the drawings or when directed by engineer bars that are to be epoxy doweled are to be put in holes larger than the bar diameter (1/4" larger for rebar and 1/8" larger for threaded bars). The holes shall be ten bar diameters deep for 4000 psi concrete or above and 15 bar diameters for concrete below 4000 psi and masonry. Fill holes with "Sika–Dur 31" Hi–Mod epoxy gel (or equal as approved by engineer).
All epoxy dowels and epoxy anchors are to be either threaded or deformed bars as per drawings. Apply epoxy as per manufacturer's recommendations. Mixing shall be done using a power mixer. For cold weather application gel shall be mixed at 70 degrees and kept at 40 degrees for 72 hours after application. Impact type drilling tools shall not be used for drilling holes or tightening anchors and shear bolt nuts into or through brick.
L. Top rebar in slabs and beams including top 6" of ties and column bars exposed to weather are to be epoxy coated after fabrication as per ASTM A 775–81 "Standard Specifications for Epoxy–coated Rebar." Splice length of epoxy–coated top bars shall be 1.7 times the length in Note E. Splice length for all other epoxy coated bars shall 1.5 times Note E.
M. Beam Rebars shall be spliced as follows, unless noted otherwise:
a. Top bars at midspan.
b. Bottom bars at support.

C9. Concrete tests shall be made by testing laboratory approved by the architect, with copies of all reports being mailed to the architect and the contractor. In general, one test shall be made for each 50 cubic yards of concrete, or each days' pour if less than 50 yards, or as directed by Architect. Each test shall consist of 5 cylinders of which one shall be tested at 7 days, 2 tested at 28 days, and two retained in reserve for later tests, if required. Specimens shall be made and tested in accordance with ASTM C–172, C–31 and C–39 standards. Slump and Air entrainment test shall also be made with each set of cylinders taken. Contractor shall provide the cylinders. The testing laboratory shall transport all cylinders. The owner shall pay for all tests.

C10. Before concrete is poured, check with all trades to insure proper placement of all openings, sleeves, curbs, conduits, bolts, inserts, etc. relating to work.

C11. Drypack concrete shall be one part Portland cement and one part sand with sufficient water to allow a small amount of paste to come to the surface. Use for grouting joists and beam pockets unless otherwise noted.

C12. Under steel column base plates, concrete grout shall be non–shrink with sufficient water to allow pouring. Ultimate compressive strength (f'c) at 28 days shall = 4,000 psi. Grout shall be non–metallic, meeting CRD–C621 and in accordance with the manufacturer's published specification for mixing and placing.

C13. Formwork not supporting weight of concrete, such as sides of beams, walls, columns, and similar parts of the work, may be removed after cumulatively curing at not less than 50 degrees for 24 hours after placing concrete, provided concrete is sufficiently hard to not be damaged by form removal operations, and provided curing and protection operations are maintained. Formwork supporting weight of concrete, such as beam soffits, joints, slabs and other structural elements, may not be removed in less than 14 days or until concrete has attained 75% of its design minimum compressive strength at 28 days. Support formwork from facing materials with structural members spaced sufficiently close to prevent deflection. Fit forms placed in successive units for continuous surfaces to be accurately aligned free from irregularities and within allowable tolerances.
Provide 1/16" camber per every 2.5 feet in concrete formwork of exposed to view concrete unless otherwise indicated by Architect/Engineer.

C14. Provide a continuous 2" x 4" key in all wall footings. Unless noted otherwise on drawings.

C15. All exposed to view concrete shall be stoned smooth while green, or as directed by Architect. No grout plaster shall be permitted.
Exposed to view concrete shall have 3/4" deep "V" groove placed vertically at 8'–0" o.c. or as directed by Architect.

C16. Protect freshly placed concrete from premature drying and excessive cold or hot temperature as per ACI 318 and maintain without drying at a relatively constant temperature for a period of time necessary for hydration of cement and proper hardening.

C17. Cold weather curing and protection requirements for concrete shall conform to the requirements of IBC Section 1905 when depositing concrete at freezing temperature or below, the concrete mix shall have a temperature of at least 50 degrees but not more than 80 degrees. The concrete shall be maintained at a temperature of not less than 50 degrees and in a moist condition for not less than 7 days after placing or as directed by the structural engineer. The use of chemicals or additives to prevent freezing will not be permitted. Contractor shall prevent frost from penetrating under footings or interior slabs on grade or postpone concrete pour. Refer also to specifications and to any directive by structural engineer for additional cold weather requirements.

C18. Architect/Engineer shall be notified 48 hours prior to pouring any concrete in order to observe reinforcing placement.

C19. All concrete shall be properly vibrated in place using internal vibrating rods.

C20. Unless otherwise noted all concrete slabs apply a liquid type membrane forming curing compound complying with ASTM C 309, type 1, class A Moisture loss shall be not more than 0.055 gr./sq. cm. applied at 200 sq.ft./gal. When temperature is 75 degrees or more during placement do not use membrane but moist cure slab for 7 days continuous minimum or see ACI Committee 305 Report "Hot Weather Concreting". Submit method of curing for approval.

V. Structural Steel

S1. All structural steel work shall comply with the latest edition of the AISC "Standard Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings", and "Code of Standard Practice". ASTM A–992 FY=50 ksi minimum specified for structural shapes, A36 steel for miscellaneous steel, and ASTM A–500 grade B for structural tubes, typical U.N.O. Cambering shall meet the standard mill practice shown on AISC "Manual of Steel Construction".

S2. Shop paint and remove all rust, oils, mill scale. Apply one coat zinc chromate 2 dry mills thick. Provide touch up field coat at all abraded and welded areas, two dry mils thick. All steel exposed to moisture conditions shall be galvanized. (Follow SSPC – Paint 20; ASTM A 780)

S3. Unless noted otherwise, all structural steel to steel bolted connections shall use 3/4" diameter high strength bolts conforming to ASTM A–325 (N) and shall have carbonized washers under the turning unit. All other bolts shall conform to ASTM A–307. A–325 bolts are to tightened by either turn of the nut method or load indicator washers. All A–325 bolt tightening shall be supervised by an independent testing agency who shall certify in writing that all bolts are properly tightened.

S4. Unless noted otherwise on plans, all steel beams shall have L/480 positive camber for 25 feet or greater spans.

S5. Welding
A. All welding to be made by certified welders using E–70 series electrodes. (For all welding of ASTM A–572 steel, E70XX electrodes shall be used and welding shall be as per AWS D1.1 "Structural Welding Code".)
B. All welders to be currently certified for all type of welds on this project under latest AWS D1.1, Structural Welding Code. Welders to have passed the Qualification Requirements within preceding 6 month period.
C. Welds made against concrete are to be done under the supervision of an approved testing agency and that fillet welds should be made in 1/8" passes 2" long at 4" o.c
D. All steel to steel connections not shown bolted which is continuous, shall be welded to develop full strength capacity of connecting members.
E. Minimum size of fillet weld (unless noted otherwise on drawings):

Material thickness of thicker part joined	Minimum size of of fillet weld
to 1/4" inclusive	1/8" all around
over 1/4" to 1/2"	3/16" all around
over 1/2" to 3/4"	1/4" all around
over 3/4" to 1 1/2"	5/16" all around

F. Unless otherwise noted, all structural steel to steel connections shall be made in such a manner to develop full shear capacity of connecting members as per AISC specifications.
G. Field paint all abraded and welded surfaces for joists and metal deck. Use SSPC – Paint 20 (Galvanic).
H. Unless otherwise indicated, all anchors welded to steel plates or angles that are embedded in masonry or concrete shall be deformed bar anchors conforming to A36 Steel or ASTM A706.
I. All deck bearing angles or plates shall have full penetration welds at splices and corners typical unless noted otherwise.
J. All full penetration welds shall be tested by x–ray or ultrasonic procedures by an independent testing agency approved by the architect. Where testing procedures are not physically possible, visual inspection before and during welding shall be done by an independent testing agency.
K. 10% of all shop and field welds shall be done under the direct supervision of an independent testing agency approved by the architect and tested by magnetic particle procedures.
L. Copies of all tests results are to be sent to structural engineer. Welds found to be defective shall be corrected at no extra cost to the owner.
M. All weld testing shall be paid for by the owner.

S6. Steel Deck
A. All metal deck shall meet requirements of Steel Deck Institute (SDI) for wide rib deck. See drawings for type of deck. Manufacture shall be a member of SDI.
B. Deck manufacturer shall have ICBO certification showing lateral shear capacities of deck equalling 1300 plf and with F (Flexibility Factor) less than 10.
C. Provide 18 gage sheet metal reinforcing at all openings through metal deck. For openings 15" and larger frame opening with angle 3 x 3 x 1/4" unless otherwise noted. End laps to occur at supports and shall have minimum lap of 2". The deck shall be attached to all supports and the side lap of adjacent units.
D. All deck splices shall occur over supporting members and shall have a minimum of 4" of flat bearing surface contact.
E. Deck Welding (unless otherwise noted):
a. Supports parallel to deck 3/4" diameter puddle welds at 12" o.c.
b. Supports perpendicular to deck 3/4" diameter at each valley.
c. Top seam welds 1 1/2" at 12" o.c. Deck shall be crimped prior to all side or top seam welding. For composite floor deck, side lap use 1/4" diameter button punch at 24" o.c..
d. Welder shall be certified as a light gage welder in accordance with AWS.
e. Use E60 electrodes.
a,b, and c are minimum deck welding, deck supplier is to indicate deck welds on shop drawings to develop stated shear capacities.
F. Unless noted otherwise on drawings all deck shall bear on and be welded to continuous angle 3 x 3 x 1/4 x cont. at all deck boundaries fastened to concrete or masonry wall with weld plates as per typical details or 3/4" diameter x 8" x 3" J–bolts at 16" o.c. Provide angle 3 1/2 x 2 1/2 x 3/16 under all changes in deck direction.
G. All continuous deck bearing angles shall have full penetration welds at splices and corners.
H. Architect/Engineer shall be notified 48 hours prior to application of roofing material in order to observe deck attachment.
I. Roof deck and joists and girders shall be designed for 17 psf uplift force minimum, U.N.O.

S7. Steel Joists
A. All open web steel joists, VS joist substitutes, loose outrigger and bridging shall be fabricated according to the specifications of the "Steel Joist Institute" (SJI) and the manufacturers shall be members of SJI joists, outriggers and VS members shall be anchored at their supports by welding unless otherwise noted. VS joist substitutes are Vulcraft "VS" series or equal. All K series joists shall bear 4 inches minimum and LH series joists and open web and girders shall bear 6 inches minimum on steel bearing angles or plates. Joist and girder supplier shall submit shop drawings showing truss joist geometry and member sizes. Joist supplier shall submit design calculations for all joists with snow drift loading, point loads, or any non–standard loading conditions.
B. Joist bearing sloping 1/4" per foot or more provide beveled steel shims welded to joist and support.
C. All joist cross bridging shall be welded at each joist and at cross over points.
D. Where steel joist are exposed, use X bridging only. Coordinate bridging location with Architect. Do not run bridging through skylights, etc., unless otherwise indicated.
E. Live Load deflection shall be limited to L over 360 unless otherwise noted.
F. Structural calculations shall be provided with a Utah P.E. stamp.

S8. Steel Studs
A. Structural steel studs shall be as specified in this note and shown on drawings with minimum effective properties.

Stud Size	Gage	Gross Area (In.2)	IXX (in.4) EFFECTIVE
6"x 1 5/8	18	.43	2.22
6"x 1 5/8	16	.54	2.76
4"x 1 5/8	16	.42	1.06
3 5/8 x 1 5/8	16	.41	.84

B. All studs shall be spaced at 16" o.c. unless noted otherwise and to be standard painted unless otherwise noted.
Fy for 16 ga. and heavier material..50 Ksi
Fy for 18 ga. and lighter material..33 Ksi
C. Unless otherwise noted all bridging to be 1 1/2" minimum x 18 ga. x continuous cold rolled channels positioned through stud punch–outs and weld attached on both sides to stud punch–out. Bridging shall be spaced at 4'–6" o.c. to match punch–outs. Where punch–outs do not line up use weld attached bridge clip angles.
D. All track to be stud size by 1 1/2" flange by 16 gage standard painted unless noted otherwise. Attach track to concrete slab at 16" o.c. using .177" diameter x 1 1/2" powder driven fasteners. Tracks and bridging to have Fy = 33,000 psi.
E. All splices of structural studs to be full strength. Use 2'–0" minimum section lapped 1'–0" above and below splice fully welded. Alternate all splices 24" minimum. Spot paint all welds after cleaning.
F. Load bearing stud walls must be fabricated with the stud ends seated against the track web. Full web and flange bearing must be provided.
G. All structural studs shall be welded to top and bottom tracks with 1/8" x 1 1/2" fillet at each stud flange and 1/8" x 3" at stud web.

S9. Steel columns and beams that are located inside concrete or masonry walls or that are in contact with the walls shall have deformed bar anchors (KSM or Nelson) welded to the steel members at the shop. The deformed bar anchors (abbreviated DBA) shall match the size and location of the horizontal or vertical wall rebar that is interrupted by the steel members and shall be of such length to lap 38 bar diameters with the concrete wall reinforcing and 48 bar diameter with the masonry wall reinforcing.

VI. General Conditions

G1. If discrepancies exist between specifications, general notes and drawings use the more expensive option.

G2. All dimensions on structural drawings shall be checked and verified against architectural drawings. All dimensions relating to existing site, buildings, installations or construction shall be field verified, all discrepancies shall be submitted to the architect. Do not proceed with fabrication and erection of materials affected until discrepancies are resolved.

G3. All omissions or discrepancies in the working drawings and or specifications shall be brought to the attention of the Architect and/or Structural Engineer before proceeding with any work involved.

G4. Shoring
A. Until all permanent members, including walls, slabs, floors and roof are in place and all connections are completed, stability of structure and all parts thereof shall be contractor's responsibility. During construction contractor shall keep construction loads within the design load limits shown on drawings. After construction is completed building owner shall keep loads on roof and floor within design limits shown on drawings.
B. Do not backfill walls until floor at top of wall is in place or adequate temporary bracing is provided.
C. Contractor shall provide shoring design calculations and drawings stamped by a Utah Registered Professional Engineer.

G5. All Construction shall be in accordance with the IBC 2003 and supplements unless higher standard is called for.

G6. Unless a more stringent requirement is specified, design all members with minimum Live Load deflection of L/360.

G7. Contractor shall be responsible for safety and protection in and around job site and or adjacent properties.

G8. Observation visits to the site by Bsumek Mu and Associates Field Representative shall neither be construed as inspection nor approval of construction.

G9. Contractor shall provide 5 sets of shop drawings for review by structural engineer for: all reinforcing bars, structural steel, glu–lam beams, wood joist and all prefab. structural items including structural calculations.

G10. All openings through floors and walls shall be verified with architectural, mechanical and electrical drawings. Do not cut openings in concrete or masonry without approval of structural engineer and architect.

G11. Contractor/Window/Door – Supplier shall provide 1/2" minimum vertical movement capability in frame system.
Window/Door– Supplier shall design for wind load specified under "Basis for Design" and shall submit professional engineer stamped design calculations showing compliance with Wind Load Capacity and vertical movement capacity.

G12. Seismic bracing of electrical, mechanical equipment, and ceiling system shall be designed by their respective supplier and stamped by a Utah Professional Engineer and submitted for design review.

G13. The appearance of all exposed structural elements shall be approved by architect or owner.
All blemishes, dents, or shipping damage in structural elements that are exposed to view shall be repaired before erection and shall be approved by the architect. All sweeps in beams joists, and girders greater than 1/2" shall be corrected. Repairs shall be made at no cost to the owner. For tolerances in wide flange shapes, follow AISC specifications.

VII. Special Inspections

Special inspections shall be done by Special Inspectors that are qualified and approved for each area of work stated below or as required by the Building Official. All special inspections shall be paid for by the OWNER. The special inspector shall observe the work assigned for conformance with the approved design drawings and specifications. The special inspector shall submit reports to the owner, the Building Official, the Contractor, Architect, and Bsumek Mu and Associates. The special inspectors shall conform to and fulfill all other responsibilities as outlined in section 1704 of the IBC 2003. The special inspector shall submit written reports of observations stating time, date, location of work and observation of work being done.

A. See note C2 for concrete above 2500 psi not requireing special inspections. Some higher strength concrete is specified for durability only.
B. Special inspections are required for the following work:
1. Concrete and reinforcing placement. Except for the following conditions:
a. For foundations satisfying requirement of IBC 2003,Table 1805.4.2.
b. Non–Structural slab on grade.
c. Site work concrete where no special hazards exist.

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UVSC HALLWAY ADDITION - DFCM
PHYSICAL EDUCATION BLDG UVSC CAMPUS
DESIGN DEVELOPMENT

Revision #	Date
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Axis Job # 0510
Owner #
Date 12-14-05
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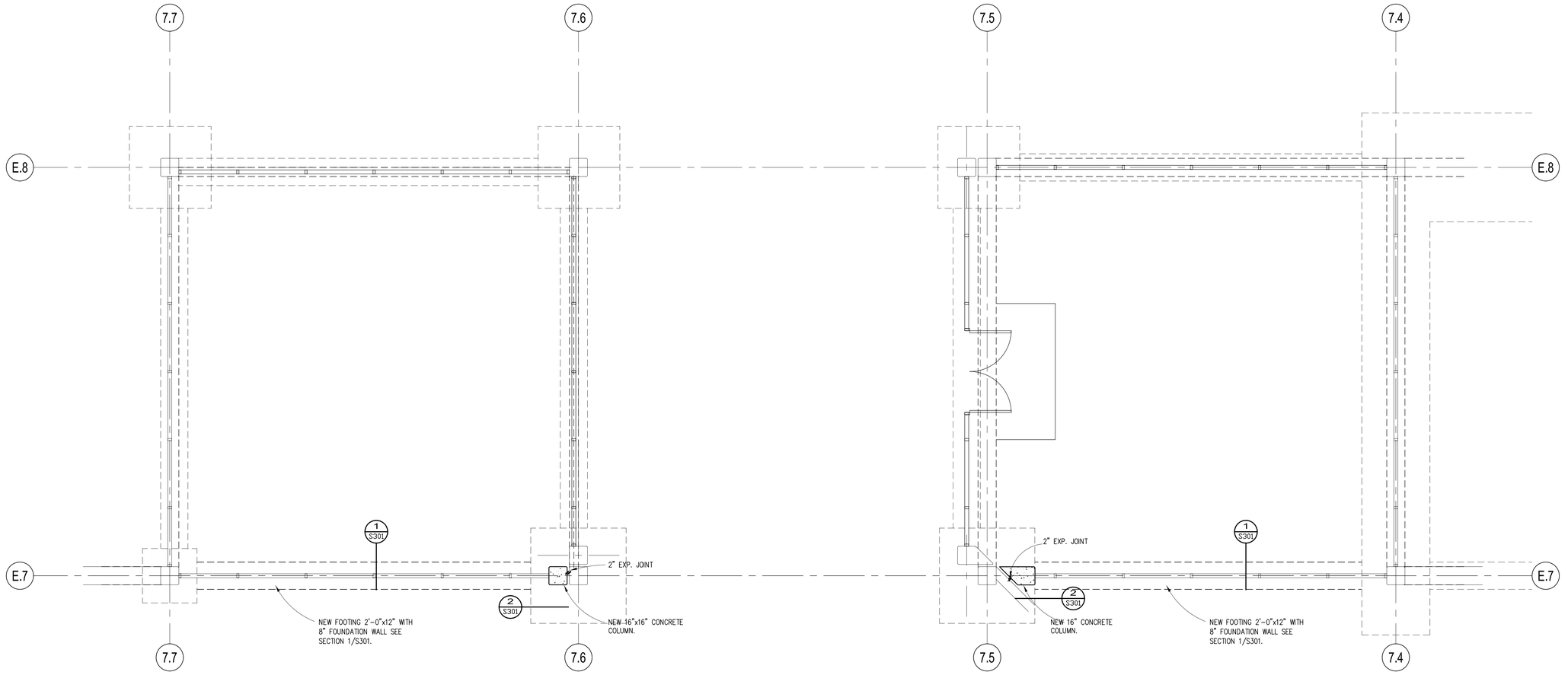
GENERAL NOTES

S001



PLAN NORTH

FOOTING AND FOUNDATION PLAN

SCALE: 1/4"=1'-0"

Plan Notes:

1. SLABS ON GRADE SHALL BE 4" UN-REINFORCED CONCRETE OVER 4" GRAVEL, COORDINATE ALL TOP OF SLAB ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
2. COORDINATE ALL DIMENSIONS, WALL LOCATIONS AND BUILDING FINISHES WITH ARCHITECT DRAWINGS.
3. ALL EXTERIOR FOOTINGS SHALL BE PLACED A MINIMUM OF 30" FROM FINISH GRADE TO BOTTOM OF FOOTING FOR FROST PROTECTION, UNLESS NOTED OTHERWISE ON SECTIONS.
4. WHERE NEW FOOTINGS TERMINATE AT EXISTING FOOTINGS ALL NEW CONTINUOUS REINFORCING SHALL EPOXY DOWEL TO EXISTING FOOTING.

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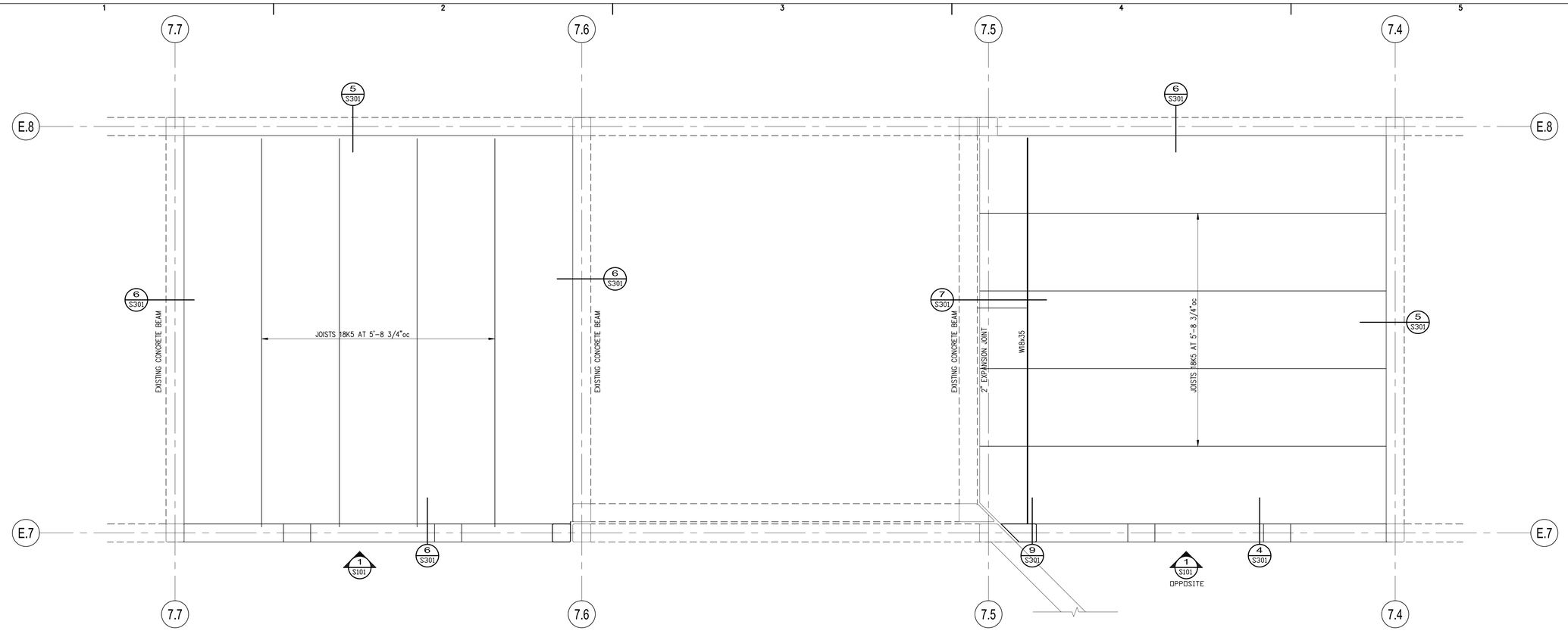
FOOTING PLAN

S101



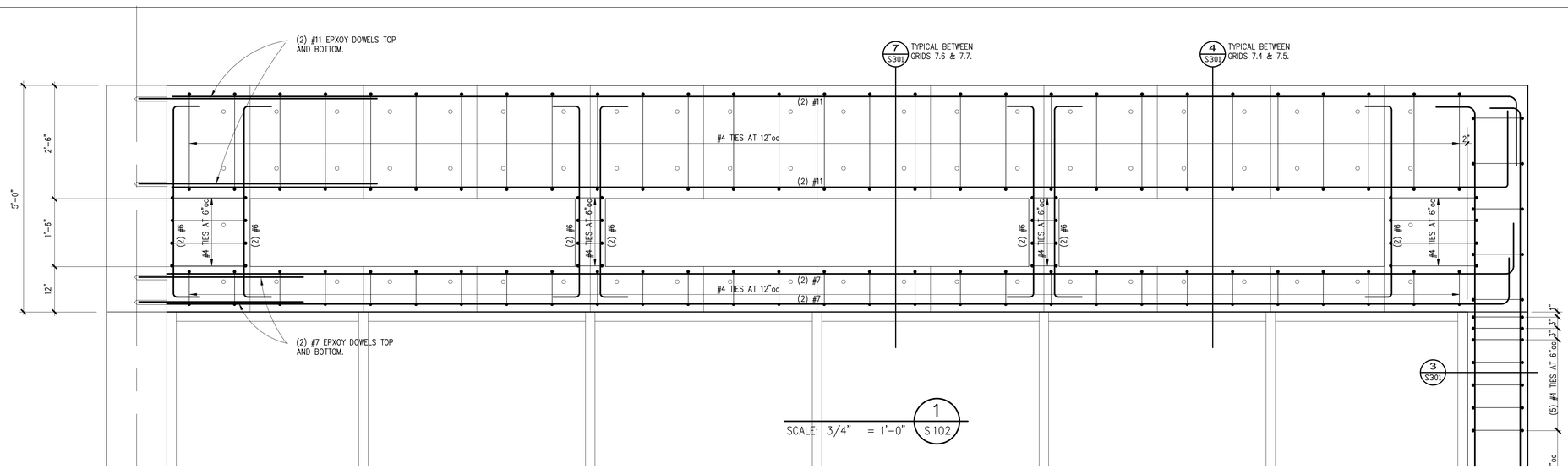
PLAN NORTH

Roof Framing Plan

SCALE: 1/8"=1'-0"

Plan Notes:

1. TYPICAL ROOF DECK SHALL BE 1_1/2"x18 GAGE METAL DECK WITH DIAPHRAGM SHEAR CAPACITY EQUAL TO 2500 PLF. SEE S0.1 FOR WELDING.
2. COORDINATE ALL ROOF PENETRATIONS WITH ARCHITECTURAL, MECHANICL, AND PLUMBING DRAWINGS. SEE SHEET S5.3 FOR FRAMING AROUND OPENINGS.



SCALE: 3/4" = 1'-0" — 1 / S102

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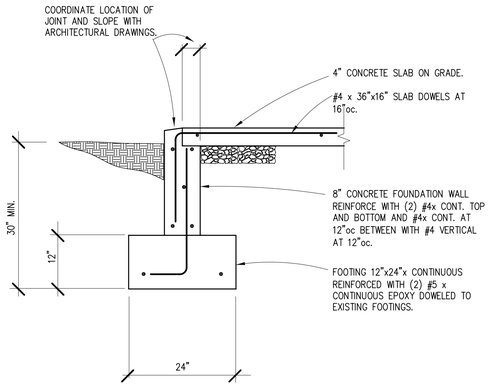
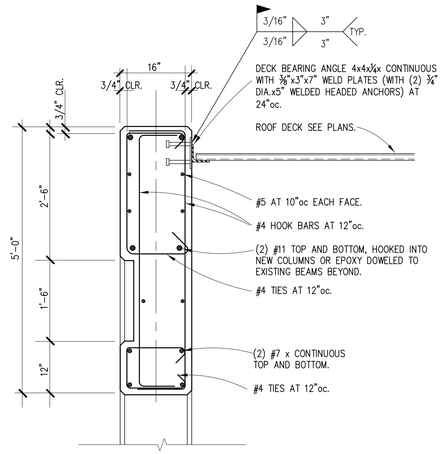
FOOTING PLAN

S102



SCALE: 3/4" = 1'-0" 1 S301



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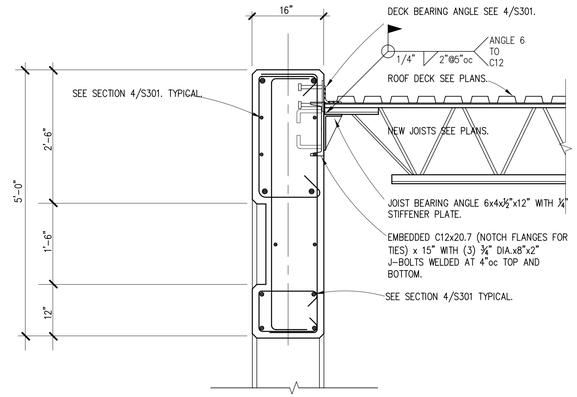

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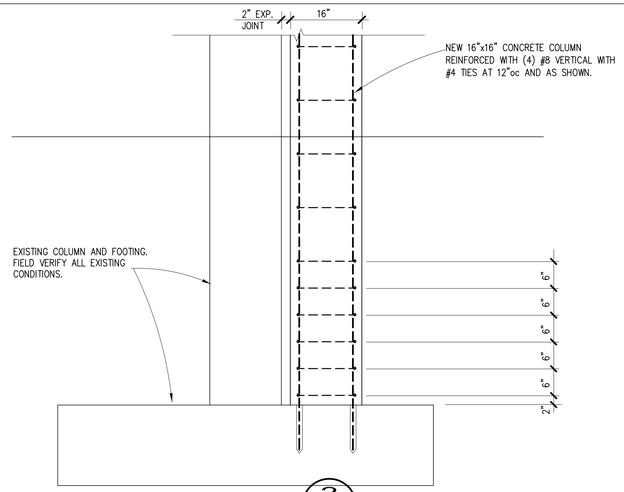

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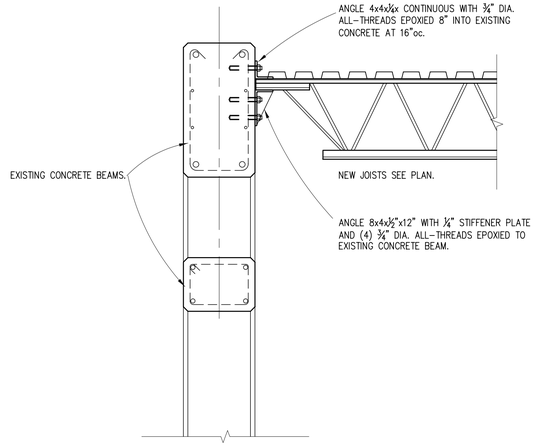

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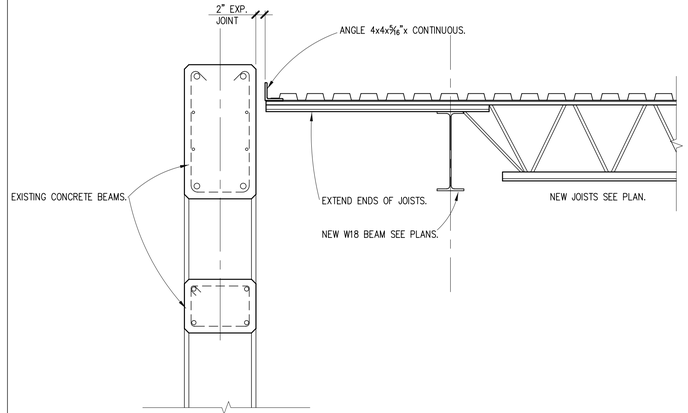
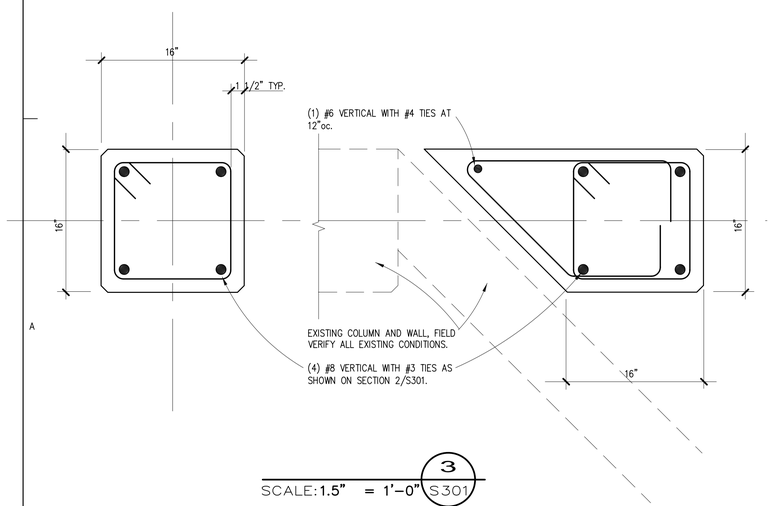


SCALE: 3/4" = 1'-0" 8 S301



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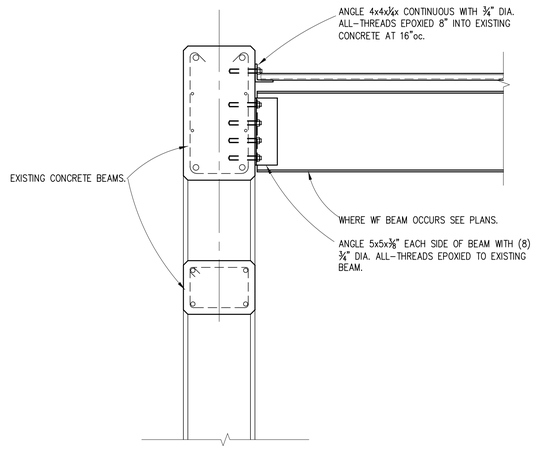

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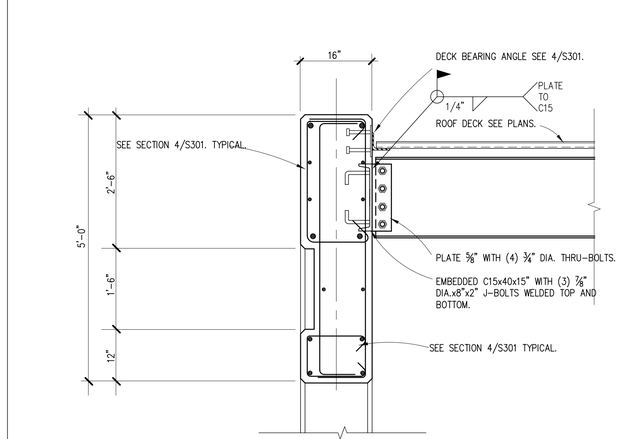

SCALE: 3/4" = 1'-0" 9 S301

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UVSC HALLWAY ADDITION - DFCM

PHYSICAL EDUCATION BLDG UVSC CAMPUS

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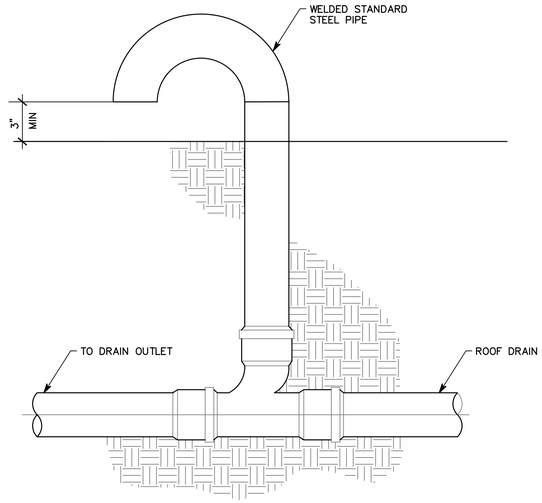
STRUCTURAL SECTIONS

S301

DIFFUSER AND GRILLE SCHEDULE

DESIGNATION	MANUF.	MODEL	DESCRIPTION	MAX SP DROP (IN. WC)
L–1	AIROLITE	K6776	WALL LOUVER , STATIONARY 6 INCH THICK 35 BLADE, 12 GA., EXT. ALUMINUM BLADES, 8 GA, EXT. ALUMINUM JAMBS. CHANNEL FRAME. BIRD SCREEN. COLOR AND FINISH TO BE SELECTED BY ARCHITECT.	N/A
LS–1	PRICE	SDS 100	LINEAR SLOT DIFFUSER, CURVED FACE, FRAME 16 FIELD VERIFY EXACT DIMENSIONS. COLOR AND FINISH TO BE SELECTED BY ARCHITECT.	0.1



3 GOOSENECK DETAIL
SCALE: NOT TO SCALE

ABBREVIATIONS

NOTE: ALL ABBREVIATIONS MAY NOT BE USED

AD	ACCESS DOOR	MCA	MINIMUM CIRCUIT AMPS
AIR	AIR CONDITION(–ING,–ED)	MFR	MANUFACTURER
COND		MIN	MINIMUM
APD	AIR PRESSURE DROP	N/A	NOT APPLICABLE
BD	BALANCING DAMPER	NC	NORMALLY CLOSED
BHP	BRAKE HORSE POWER	NC	NOISE CRITERIA
BTU	BRITISH THERMAL UNIT	NIC	NOT IN CONTRACT
BTUH	BTU/HOUR	NO	NORMALLY OPEN
CFH	CUBIC FEET PER HOUR	NPSH	NET POSITIVE SUCTION
CFM	CUBIC FEET PER MINUTE		HEAD
CLG	COOLING	NTS	NOT TO SCALE
COMP	COMPONENT	OA	OUTSIDE AIR
COND	CONDENS(–ER, –ING, –ATION)	OD	OUTSIDE DIAMETER
		OZ	OUNCE
CV	CONTROL VALVE	PD	PRESSURE DROP OR DIFFERENCE
CW	COLD WATER		
DIA	DIAMETER	PG	PROPYLENE GLYCOL
DISCH	DISCHARGE	PH	PHASE
DP	DEPTH OR DEEP	PPM	PARTS PER MILLION
DB	DRY BULB TEMPERATURE	PRESS	PRESSURE
(E)	EXISTING	PSF	POUNDS PER SQUARE FOOT
EER	ENERGY EFFICIENCY RATIO		
EFF	EFFICIENCY	PSI	POUNDS PER SQUARE INCH
EG	ETHYLENE GLYCOL		
ELEC	ELECTRIC	PSIA	PSI ABSOLUTE
ELEV	ELEVATION	PSIG	PSI GAUGE
ENT	ENTERING	R	THERMAL RESISTANCE
EVAP	EVAPORAT(–E, –ING, –ED, –OR)	RA	RETURN AIR
		RECIRC	RECIRCULATE
EWT	ENTERING WATER TEMPERATURE	REFR	REFRIGERATION
		REQD	REQUIRED
EXT	EXTERNAL	RLA	RATED ––– AMPS
(F)	FUTURE	RPM	REVOLUTIONS PER MINUTE
F	FAHRENHEIT	RW	RAINWATER
FC	FLEXIBLE CONNECT(–OR, –ION)	SA	SUPPLY AIR
		SC	SHADING COEFFICIENT
FD	FIRE DAMPER	SCFM	STANDARD CUBIC FEET PER MINUTE
FLA	FULL LOAD AMPS		
FPI	FINS PER INCH	SCW	SOFT COLD WATER
FPM	FEET PER MINUTE	SF	SAFETY FACTOR
FPS	FEET PER SECOND	SH	SENSIBLE HEAT
FSD	FIRE SMOKE DAMPER	SL	SEA LEVEL
FT	FEET	SP	STATIC PRESSURE
GAL	GALLON(S)	SPECS(S)	SPECIFICATION(S)
GPH	GALLONS PER HOUR	SQ	SQUARE
GPM	GALLONS PER MINUTE	STD	STANDARD
HD	HEAD	STM	STEAM
HG	MERCURY	TEMP	TEMPERATURE
HR	HOUR	TD	TEMP. DROP OR DIFF.
HT	HEIGHT	THERM	THERMAL
HTG	HEATING	TOT	TOTAL
HP	HORSE POWER	TSTAT	THERMOSTAT
HW	HOT WATER	V	VOLT
HZ	HERTZ(FREQUENCY)	VAC	VACUUM
ID	INSIDE DIAMETER	VAV	VARIABLE AIR VOLUME
IN	INCH	VEL	VELOCITY
KW	KILOWATT	VENT	VENT, VENTILATION
LAT	LEAVING AIR TEMPERATURE	VERT	VERTICAL
		VFD	VARIABLE FREQUENCY DRIVE
LBS	POUNDS		
LG	LENGTH	VOL	VOLUME
LH	LATENT HEAT	WC	WATER COLUMN
LRA	LOCKED ROTOR AMPS	WG	WATER GAUGE
LVG	LEAVING	WPD	WATER PRESSURE DROP
LWT	LEAVING WATER TEMPERATURE	WTR	WATER
		WT	WEIGHT
MAX	MAXIMUM	WB	WET BULB TEMP
MBH	THOUSAND BTU PER HOUR	YR	YEAR

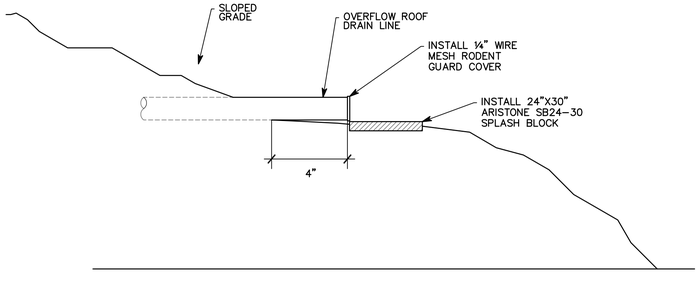
SYMBOL LEGEND

SYMBOL	DESCRIPTION
DUCTWORK	

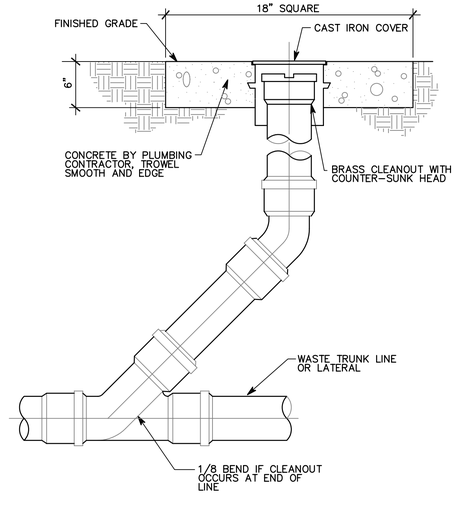
SINGLE LINE	DOUBLE LINE	DESCRIPTION
		RECTANGULAR SUPPLY DUCT UP
		RECTANGULAR SUPPLY DUCT DOWN
		RECTANGULAR RETURN DUCT UP
		RECTANGULAR RETURN DUCT DOWN
		RECTANGULAR EXHAUST DUCT UP
		RECTANGULAR EXHAUST DUCT DOWN
		ROUND DUCT UP
		ROUND DUCT DOWN
		ACCOUSTICALLY LINED RECTANGULAR DUCT
		90° RECTANGULAR ELBOW WITH TURNING VANES
		90° RADIUS ELBOW R=1.5
		DUCT SIZE OR SHAPE TRANSITION
		OPPOSED BLADE BALANDING DAMPER (O.B.D.) IN RECT DUCT
		EXISTING DUCT
		DUCT TO BE REMOVED

SYMBOL LEGEND

SYMBOL	DESCRIPTION
PLUMBING PIPING	
	SOIL, WASTE – ABOVE GRADE
	SOIL, WASTE – BELOW GRADE
RW	RAINWATER – ABOVE GRADE
RW	RAINWATER – BELOW GRADE
ORW	OVERFLOW RAINWATER ABOVE GRADE
ORW	OVERFLOW RAINWATER BELOW GRADE
SD	STORM DRAIN
(E)	EXISTING PIPE
(E)	EXISTING PIPE TO BE REMOVED



2 OVERFLOW DRAIN LINE DETAIL
SCALE: NOT TO SCALE



1 CLEANOUT TO GRADE DETAIL
SCALE: NOT TO SCALE

GENERAL MECHANICAL NOTES

1. DO NOT ROUTE DUCTS AND PIPES ABOVE ELECTRICAL PANELS. ALL ELECTRICAL PANELS MUST HAVE CLEAR ACCESS SPACE IN FRONT OF PANEL 4'–0" DEEP AND 6'–6" HIGH. DO NOT ROUTE DUCTS AND PIPES IN ELECTRICAL ROOMS, EXCEPT DUCTS AND PIPES SERVING THE ROOM.
2. ALL DUCT DIMENSIONS ARE INSIDE FREE AREA DIMENSIONS. ADJUST SHEET METAL DIMENSION FOR LINED DUCT.
3. IF CONTRACTOR ENCOUNTERS MATERIAL WHICH MAY CONTAIN ASBESTOS IMMEDIATELY STOP WORK IN THIS AREA AND NOTIFY THE OWNER.
4. PROVIDE CEILING ACCESS PANELS AS REQUIRED WHERE MECHANICAL EQUIPMENT, VALVES, VAV BOXES, FIRE DAM– PERS, ETC. ARE LOCATED ABOVE INACCESSIBLE CEILINGS.
5. ALL DUCT AND FLUE PENETRATIONS THRU 1 HOUR ROOF ASSEMBLY TO BE ENCLOSED WITH 2 SHEET ROCK LAYERS FROM SHEET ROCK AT BOTTOM OF ROOF TRUSSES TO ROOF DECK.
6. STEEL ROOF DECK SHALL NOT BE USED TO SUPPORT LOADS FROM PIPING, DUCTWORK OR EQUIPMENT, UNLESS NOTED OTHERWISE. HANGER LOADS LESS THAN 50 LBS. MAY BE HUNG FROM THE STEEL ROOF DECK IN CASES WHEN HANGING FROM THE STEEL ROOF DECK CANNOT BE AVOIDED; THE ATTACHMENT METHOD MUST DISTRIBUTE THE LOAD ACCROSS THE DECK AS APPROVED BY THE STRUCTURAL ENGINEER.

MECHANICAL SHEET INDEX

SYMBOL LEGEND

SYMBOL	DESCRIPTION
REFERENCE AND LINE SYMBOLS	
# / SHEET	DETAIL INDICATOR: # INDICATES DETAIL NUMBER, SHEET INDICATES DRAWING SHEET WHERE DETAIL IS SHOWN.
# / SHEET	ELEVATION OR SECTION INDICATOR, EXTERIOR: # INDICATES ELEVATION OR SECTION NUMBER, SHEET INDICATES DRAWING SHEET WHERE ELEVATION OR SECTION IS SHOWN.
# / SHEET	ELEVATION OR SECTION INDICATOR, INTERIOR: # INDICATES ELEVATION OR SECTION NUMBER, SHEET INDICATES DRAWING SHEET WHERE ELEVATION OR SECTION IS SHOWN.
100	ROOM OR SPACE NUMBER.
1	KEYNOTE INDICATOR.
	REVISION INDICATOR.
CU–1	EQUIPMENT INDICATOR.
P–	PLUMBING FIXTURE INDICATOR.
TYPE / CFM / SIZE	DIFFUSER/GRILLE INDICATOR.
TYPE / SIZE	DIFFUSER/GRILLE INDICATOR.
	BREAK, STRAIGHT
	BREAK, ROUND.
MATCH LINE / SEE XX/X–XXX	MATCH LINE INDICATOR
	HIDDEN FEATURES LINE: HIDDEN, THIN LINE.
	CONTRACT LIMIT LINE: DASHDOT, WIDE LINE.
	NEW CONNECTION POINT TO EXISTING

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MECHANICAL SYMBOLS LEGEND AND SHEET INDEX

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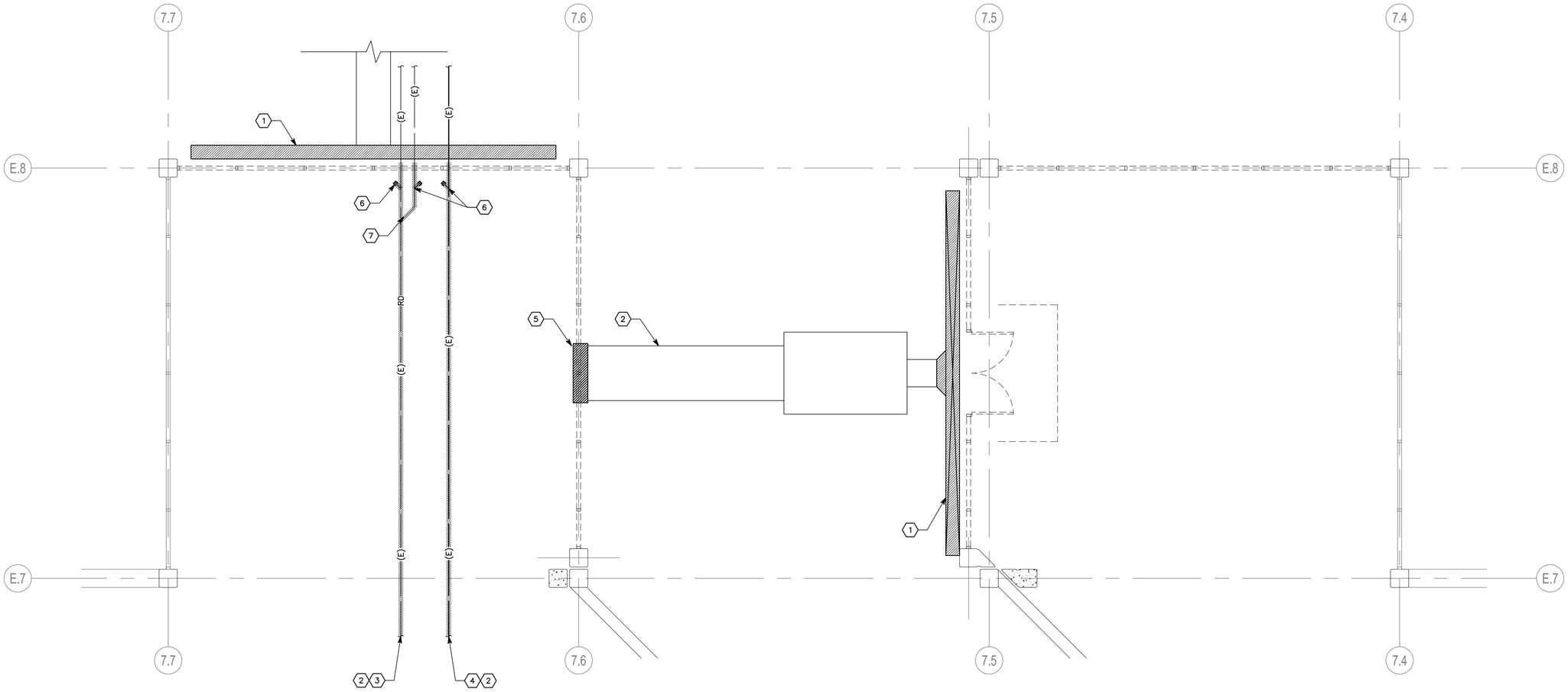

1 MECHANICAL DEMOLITION FLOOR PLAN

SCALE: 1/4"=1'-0"

PROJECT NORTH

SHEET KEYNOTES

1. REMOVE EXISTING SUPPLY AIR DUCT.
2. DEDUCTIVE ALTERNATE: IF EXISTING PIPE MATERIAL MEETS CURRENT CODE IT DOES NOT NEED TO BE REMOVED. (SEE IPC 2003 TABLE 702.2 FOR CODE COMPLIANT UNDERGROUND PIPE MATERIALS).
3. REMOVE EXISTING STORM DRAIN LINE.
4. REMOVE EXISTING WASTE LINE.
5. REMOVE EXISTING OUTSIDE AIR LOUVER.
6. REMOVE EXISTING CLEANOUTS.
7. OVERFLOW ROOF DRAIN WILL BE DETACHED AND EXTEND TO COMPLY WITH CODE.

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MECHANICAL
DEMOLITION
FLOOR PLAN

MD101

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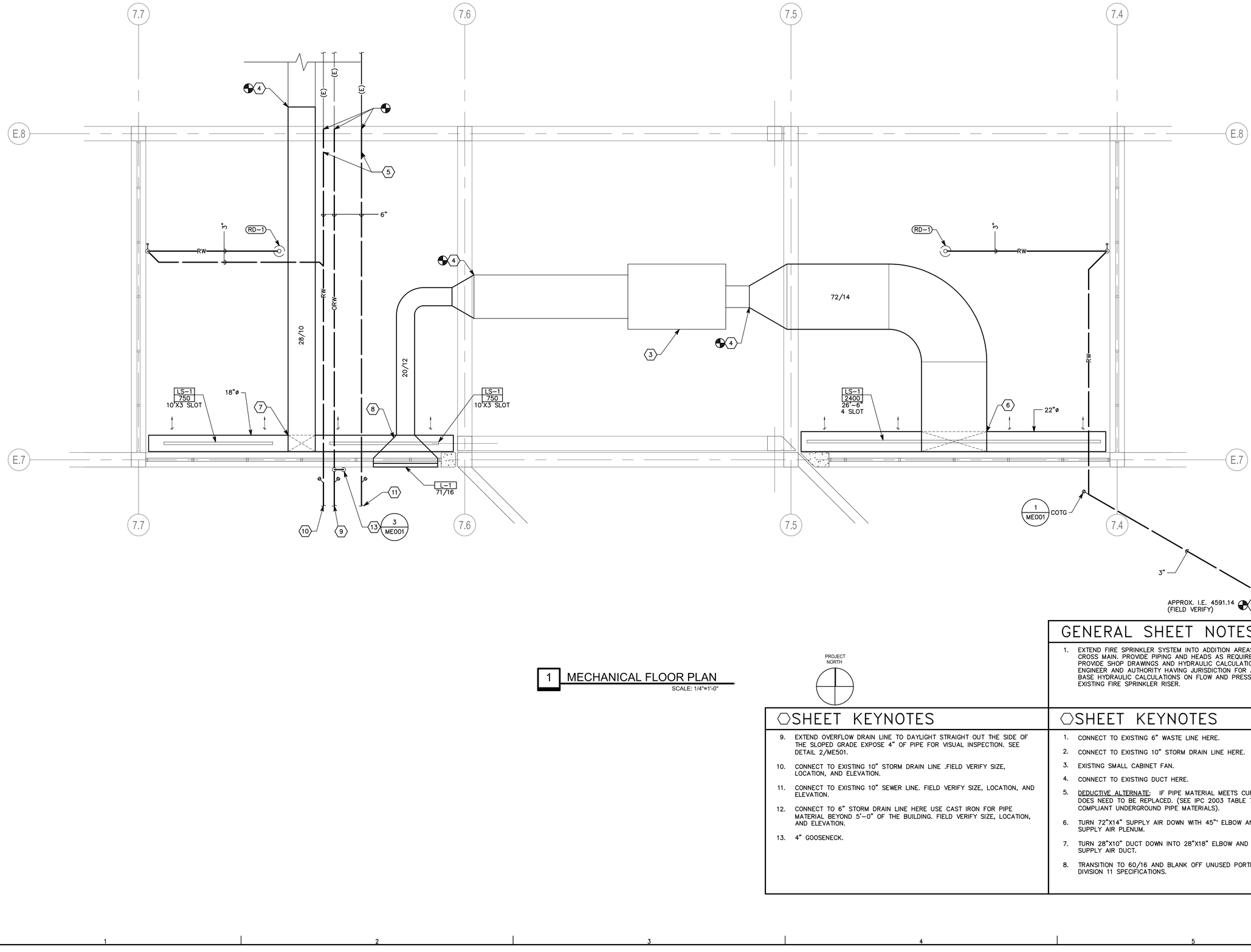

1 MECHANICAL FLOOR PLAN

SCALE: 1/4"=1'-0"

PROJECT NORTH

GENERAL SHEET NOTES

1. EXTEND FIRE SPRINKLER SYSTEM INTO ADDITION AREAS FROM EXISTING CROSS MAIN. PROVIDE PIPING AND HEADS AS REQUIRED BY NFPA 13. PROVIDE SHOP DRAWINGS AND HYDRAULIC CALCULATIONS. SUBMIT TO ENGINEER AND AUTHORITY HAVING JURISDICTION FOR APPROVAL. DESIGN BASE HYDRAULIC CALCULATIONS ON FLOW AND PRESSURE AT BASE OF EXISTING FIRE SPRINKLER RISER.

⬡SHEET KEYNOTES

9. EXTEND OVERFLOW DRAIN LINE TO DAYLIGHT STRAIGHT OUT THE SIDE OF THE SLOPED GRADE EXPOSE 4" OF PIPE FOR VISUAL INSPECTION. SEE DETAIL 2/ME501.
10. CONNECT TO EXISTING 10" STORM DRAIN LINE .FIELD VERIFY SIZE, LOCATION, AND ELEVATION.
11. CONNECT TO EXISTING 10" SEWER LINE. FIELD VERIFY SIZE, LOCATION, AND ELEVATION.
12. CONNECT TO 6" STORM DRAIN LINE HERE USE CAST IRON FOR PIPE MATERIAL BEYOND 5'-0" OF THE BUILDING. FIELD VERIFY SIZE, LOCATION, AND ELEVATION.
13. 4" GOOSENECK.

⬡SHEET KEYNOTES

1. CONNECT TO EXISTING 6" WASTE LINE HERE.
2. CONNECT TO EXISTING 10" STORM DRAIN LINE HERE.
3. EXISTING SMALL CABINET FAN.
4. CONNECT TO EXISTING DUCT HERE.
5. DEDUCTIVE ALTERNATE: IF PIPE MATERIAL MEETS CURRENT CODE ITEMS IT DOES NEED TO BE REPLACED. (SEE IPC 2003 TABLE 702.2 FOR CODE COMPLIANT UNDERGROUND PIPE MATERIALS).
6. TURN 72"X14" SUPPLY AIR DOWN WITH 45°+ ELBOW AND CONNECT TO 22"ø SUPPLY AIR PLENUM.
7. TURN 28"X10" DUCT DOWN INTO 28"X18" ELBOW AND CONNECT INTO 18"ø SUPPLY AIR DUCT.
8. TRANSITION TO 60/16 AND BLANK OFF UNUSED PORTION OF LOUVER. SEE DIVISION 11 SPECIFICATIONS.

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MECHANICAL FLOOR PLAN

MH101

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FIXTURE SCHEDULE

TYPE	DESCRIPTION	CATALOG NUMBER	VOLTS	LAMPS
A	AIRCRAFT CABLE PENDANT MOUNTED LINEAR FLUORESCENT 1/2 X 1/2 ACRYLIC EGGCRATE LOUVER BLACK PAINT; ELECTRONIC BALLASTS; LENGTHS AS SHOWN ON DRAWINGS; PENDANT LENGTH PER ARCHITECT	LINEAR SQ661-D-2-277-ET8-277-SPL-SXX-BLACK-FU-20	277	(2) F32 T8 835 PER 4'

DEMOLITION NOTES

1. COORDINATE ALL NEW ELECTRICAL EQUIPMENT REQUIREMENTS AND MAKE CONNECTION TO EXISTING SYSTEMS. THIS INCLUDES LIGHTING, POWER, SIGNAL, RACEWAY AND OTHER SYSTEMS INCLUDED UNDER DIVISION 16.
2. RELOCATE, REWIRE AND/OR RECONNECT EXISTING ELECTRICAL DEVICES AND/OR EQUIPMENT THAT FOR ANY REASON OBSTRUCTS CONSTRUCTION.
3. CONCEAL ALL RACEWAY AND WIRING IN EXISTING WALLS, CEILINGS, FLOORS, ETC. EXCEPT WHERE THE USE OF SURFACE METAL RACEWAYS (E.G. WIRE MOLD) IS INDICATED ON DRAWINGS OR IN SPEC.
4. LEAVE ALL EXISTING EQUIPMENT, IN PORTIONS OF THE BUILDING NOT BEING REMODELED, IN WORKING CONDITION. RESTORE ALL INTERRUPTED BRANCH CIRCUITS, FEEDERS, ETC. TO WORKING CONDITION.
5. EXISTING RACEWAYS MAY BE REUSED (IN PLACE) WHERE POSSIBLE, AND WHERE IN COMPLIANCE WITH THE SPECIFICATIONS AND THE INTENT OF THE CONTRACT DOCUMENTS. INSURE INTEGRITY OF EXISTING RACEWAY BEFORE REUSE.
6. REMOVE ALL RACEWAYS, CONDUCTORS, BOXES, DEVICES, EQUIPMENT, ETC. THAT ARE NOT TO BE REUSED.
7. REMOVE EXISTING LIGHT FIXTURES WHICH ARE NOT TO BE REUSED, PLACE IN CARTON, LABEL APPROPRIATELY, AND RETURN TO OWNER, OR PROPERLY DISPOSE OF FIXTURES THAT THE OWNER CHOOSES NOT TO KEEP.
8. DO NOT PENETRATE STRUCTURAL ELEMENTS OF FLOORS, WALLS, CEILINGS, ROOFS, ETC.
9. DISCONNECT AND RECONNECT ANY/ALL FIXTURES, DEVICES, EQUIPMENT, ETC. REQUIRED FOR PROPER COMPLETION OF THE WORK.

GENERAL NOTES

1. CONSULT ARCHITECTURAL REFLECTED CEILING PLANS FOR EXACT LOCATION OF ALL LIGHTING FIXTURES.
2. VERIFY ALL EQUIPMENT DIMENSIONS AND LOCATIONS BEFORE BEGINNING ROUGH IN. CONSULT ALL APPLICABLE CONTRACT DRAWINGS AND SHOP DRAWINGS TO INSURE NEC CODE CLEARANCES REQUIRED AROUND ALL ELECTRICAL EQUIPMENT.
3. CONTRACTOR SHALL VERIFY ALL ELECTRICAL LOADS (VOLTAGE, PHASE, CONNECTION REQUIREMENTS, ETC.) OF EQUIPMENT FURNISHED UNDER DIVISION 15 WITH APPROVED MECHANICAL SHOP DRAWINGS BEFORE BEGINNING ROUGH IN.
4. SEE SECTION 16510 OF THE SPECIFICATION REQUIRED COORDINATION MEETINGS WITH MECHANICAL AND CEILING CONTRACTORS.
5. SEE APPLICABLE SHOP DRAWINGS FOR ROUGH IN LOCATION OF ALL EQUIPMENT, WIRING DEVICES, ETC. WHERE APPLICABLE MOUNT ALL WIRING DEVICES ABOVE BACK SPLASH EXCEPT THOSE SERVING UNDER COUNTER EQUIPMENT.
6. SEE SPECIFICATION FOR ENERGY SAVING LAMP AND BALLAST REQUIREMENTS.
7. FINISHES OF ALL LIGHT FIXTURES SHALL BE AS SELECTED BY ARCHITECT.
8. THE ELECTRICAL CONTRACTOR SHALL NOTIFY AND COOPERATE WITH THE MECHANICAL CONTRACTOR SUCH THAT NO PIPING, DUCTS, OR EQUIPMENT FOREIGN TO THE OPERATION OF THE ELECTRICAL EQUIPMENT SHALL BE PERMITTED TO BE INSTALLED IN, ENTER OR PASS THRU ELECTRICAL ROOMS OR SPACES, OR ABOVE OR BELOW ELECTRICAL EQUIPMENT IN OTHER AREAS.
9. ELECTRICAL BOXES SHALL NOT BE LOCATED IN MASONRY COLUMNS IN BRICK WALLS OR IN GROUTED CELLS ADJACENT TO OPENINGS. COORDINATE LOCATION OF BOXES WITH MASONRY CONTRACTOR.
10. ALL PENETRATIONS OF FIRE RATED FLOORS, WALLS, AND CEILINGS SHALL BE SEALED WITH APPROVED MATERIAL TO MAINTAIN FIRE RATING OF SURFACE PENETRATED.
11. CIRCUITS EXTENDING OVER 70' FOR 120 VOLT AND 165' FOR 277 VOLT 20 AMP CIRCUITS SHALL BE RUN WITH MINIMUM #10 CONDUCTORS.

INDEX OF ELECTRICAL DRAWINGS

ELECTRICAL SYMBOL SCHEDULE

1. SEE FIXTURE SCHEDULE FOR TYPE, MOUNTING AND WATTAGE.
2. HEIGHT MEASURED TO CENTER LINE OF THE BOX FROM THE FINISH FLOOR.
3. REFER TO DRAWINGS FOR DIRECTIONAL ARROWS.
4. SUBSCRIPT KEYS SWITCH TO FIXTURES CONTROLLED.
5. NEMA TYPE 'ND' NON-FUSED UNLESS NOTED 'F' (FUSED). USE 'HD' 480 V.
6. HEIGHT TO BE THE LOWER OF EITHER 80" A.F.F. OR 6" BELOW CEILING.
7. PROVIDE H.O.A. AND S.S. PUSHBUTTONS AS REQUIRED.
8. DOUBLE ARROWS DENOTE A DOUBLE FACE UNIT.
9. COORDINATE WITH MILLWORK SHOP DRAWINGS AND ELEVATIONS FOR HEIGHT.
10. SUBSCRIPT DENOTES NEMA CONFIGURATION.
11. HEIGHT MEASURED TO BOTTOM OF THE BOX FROM FINISH FLOOR.

STANDARD MOUNTING HEIGHT UNLESS OTHERWISE NOTED ON PLANS

SYMBOL	DESCRIPTION	MOUNTING HEIGHT	NOTES
	ONE CIRCUIT, TWO WIRE HOME RUN TO PANEL		
	2 CIRCUIT, 3 WIRE, COMMON NEUTRAL HOME RUN		
	3 CIRCUIT, 4 WIRE, COMMON NEUTRAL HOME RUN		
	CONDUIT UP		
	CONDUIT DOWN		
	CONDUIT STUB LOCATION	CAP CONDUIT	
	CABLE TRAY	AS NOTED	
	FLUORESCENT LIGHT FIXTURE	AS NOTED	1
	FLUORESCENT EGRESS LIGHT FIXTURE	AS NOTED	UNSWITCHED
	CEILING MOUNTED EXIT LIGHT	CEILING	1.3.8.
S	SINGLE POLE SWITCH	+4'-0"	2.
	OCCUPANCY SENSOR	CEILING	
P	POWER PACK	CEILING	SEE DIAGRAM, SPEC.
	DUPLEX RECEPTACLE	+16" OR AS NOTED	9. 11.
WP	WEATHERPROOF RECEPTACLE	+24" OR AS NOTED	2. 9.
	MULTIPLE SERVICE FLOOR BOX	FLOOR	
	DATA OUTLET	+16" OR AS NOTED	9. 11.
	PANEL BOARD	TOP AT +6'-0"	
H	FIRE ALARM SIGNAL HORN/STROBE	+6'-8"	6.
842	ARCHITECTURAL ROOM NUMBER		
A	LIGHT FIXTURE (LETTER DESIGNATES TYPE)		

Axis Architects
610 "I" STREET SALT LAKE CITY UTAH 84103 P 355-3003 F 355-6578

LICENSED PROFESSIONAL ENGINEER
B. ELAINE FAWSON
No. 295587
STATE OF UTAH

UVSC HALLWAY ADDITION - DFCM

PHYSICAL EDUCATION BLDG UVSC CAMPUS

Revision #	Date

Axis Job # 0510
Owner #
Date 11-29-05
Drawn BNA
Checked EF

SYMBOLS, SCHEDULES AND NOTES

EG101

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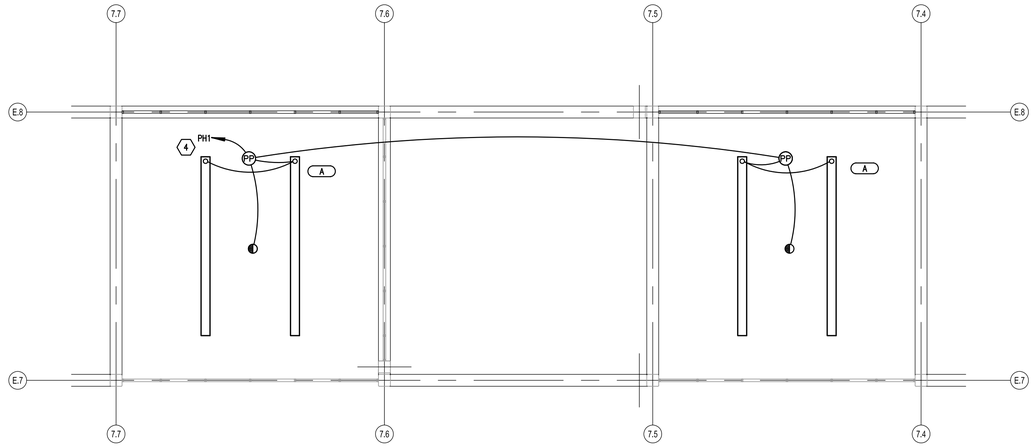
SHEET KEYNOTES

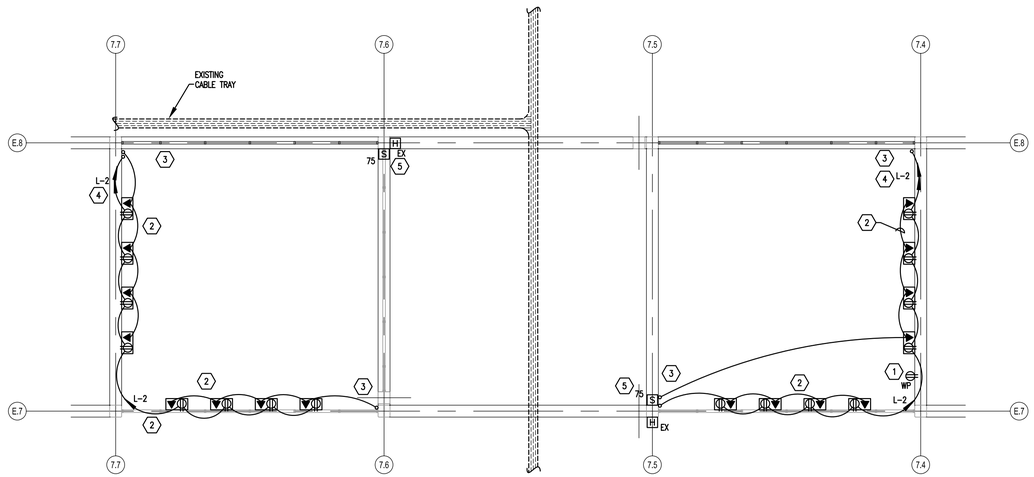
1. REMOVE EXISTING WEATHERPROOF OUTLET AND COVER PLATE AND REPLACE WITH NEW 20 AMP OUTLET AND STAINLESS STEEL COVER PLATE.
2. PROVIDE 1" CONDUIT BETWEEN EACH FLOOR BOX FOR DATA; RUN CONDUIT UP THE WALL AT LOCATION SHOWN AND THEN STUB TO CABLE TRAY. PROVIDE PULL STRING IN ALL RACEWAYS.
3. PAINT WALL MOUNTED CONDUITS TO MATCH EXISTING WALL SURFACE PER ARCHITECT'S INSTRUCTIONS. RUN CEILING CONDUIT IN UVSC STANDARD ELECTRICAL ZONE AREA, AND PAINT CONDUIT TO MATCH EXISTING ELECTRICAL CONDUITS.
4. CIRCUIT TO 20 AMP 1 POLE CIRCUIT BREAKERS IN EXISTING PANEL INDICATED. PANELS ARE LOCATED IN MECHANICAL MEZZANINE ABOVE. FIELD VERIFY EXACT LOCATION. PROVIDE A SEPARATE NEUTRAL FOR EACH CIRCUIT.
5. ADD A NEW 75 CANDELA STROBE ON COLUMN, AND TIE TO EXISTING FIRE ALARM INDICATING LOOP. STROBE SHALL BE UL LISTED TO BE USED WITH EXISTING FIRE ALARM SYSTEM. INSTALL STROBE IN SURFACE WIREMOLD BOX. WIRING SHALL BE INSTALLED IN WIREMOLD SURFACE RACEWAY. MINIMUM WIRE SIZE SHALL BE #14 AWG OF THE TYPE RECOMMENDED BY THE FIRE ALARM MANUFACTURER.



PLAN NORTH

LIGHTING PLAN
1/8" = 1'-0"

0' 4' 8' 16'



PLAN NORTH

POWER PLAN
1/8" = 1'-0"

0' 4' 8' 16'

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ELECTRICAL FLOOR PLAN

EE101

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